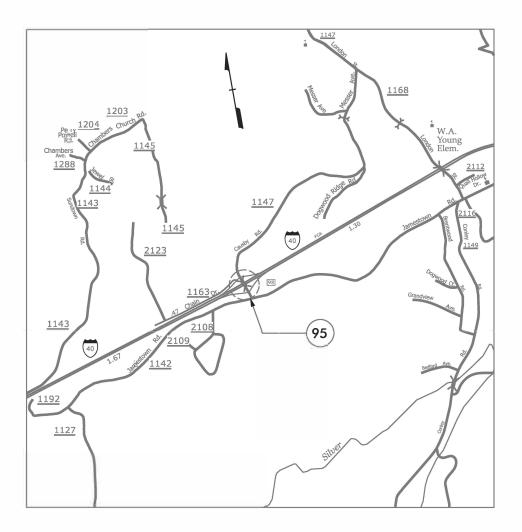


STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

BURKE COUNTY

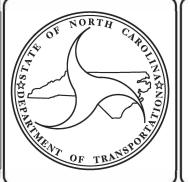
LOCATION: BRIDGE No. 110095 ON SR-1147 (CAUSBY ROAD) OVER I-40

TYPE OF WORK: BRIDGE REHABILITATION: REMOVAL OF EXISTING REINFORCED CONCRETE DECK GIRDERS SUPERSTRUCTURE AND REPLACEMENT WITH CORED SLABS, PARTIAL SUBSTRUCTURE REPLACEMENT, SHOTCRETE AND CONCRETE REPAIRS TO SUBSTRUCTURE



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PROJEC



DESIGN DATA

BURKE COUNTY BRIDGE No. 110095 ADT 2016 = 2,200

PROJECT LENGTH

BURKE COUNTY BRIDGE No. 110095 = 0.039 MILE

STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS			
N.C.	41	1665.13A	1				
STATE	PROJ. NO.	F. A. PROJ. NO.	OESCRUP	TION			
4166	65.13A		P.E				
4166	5.13A		CON	CONST.			
			_				
			-				
		·					

DIVISION STRUCTURES 1000 BI	d in the Office of: OF HIGHWAYS MANAGEMENT UNIT RCH RIDGE DR. EIGH, N.C. 27610
2018 STANDARD SPECIFICATIONS	
LETTING DATE :	ADAM COLE, P.E. PROJECT ENGINEER
JUNE 15, 2022	AMBER LEE, P.E. PROJECT DESIGN ENGINEER
JI.	

STANDARD NOTES

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SO.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SO.IN.
EOUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUITMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REOUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS, IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOITOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS, DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{1}{6}$ " \varnothing SHEAR STUDS FOR THE $\frac{3}{4}$ " \varnothing STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{6}$ " \varnothing STUDS FOR 4 - $\frac{3}{4}$ " \varnothing STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{6}$ " \varnothing STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \varnothing STUDS BOR THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{1}{2}$ ("IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES,ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

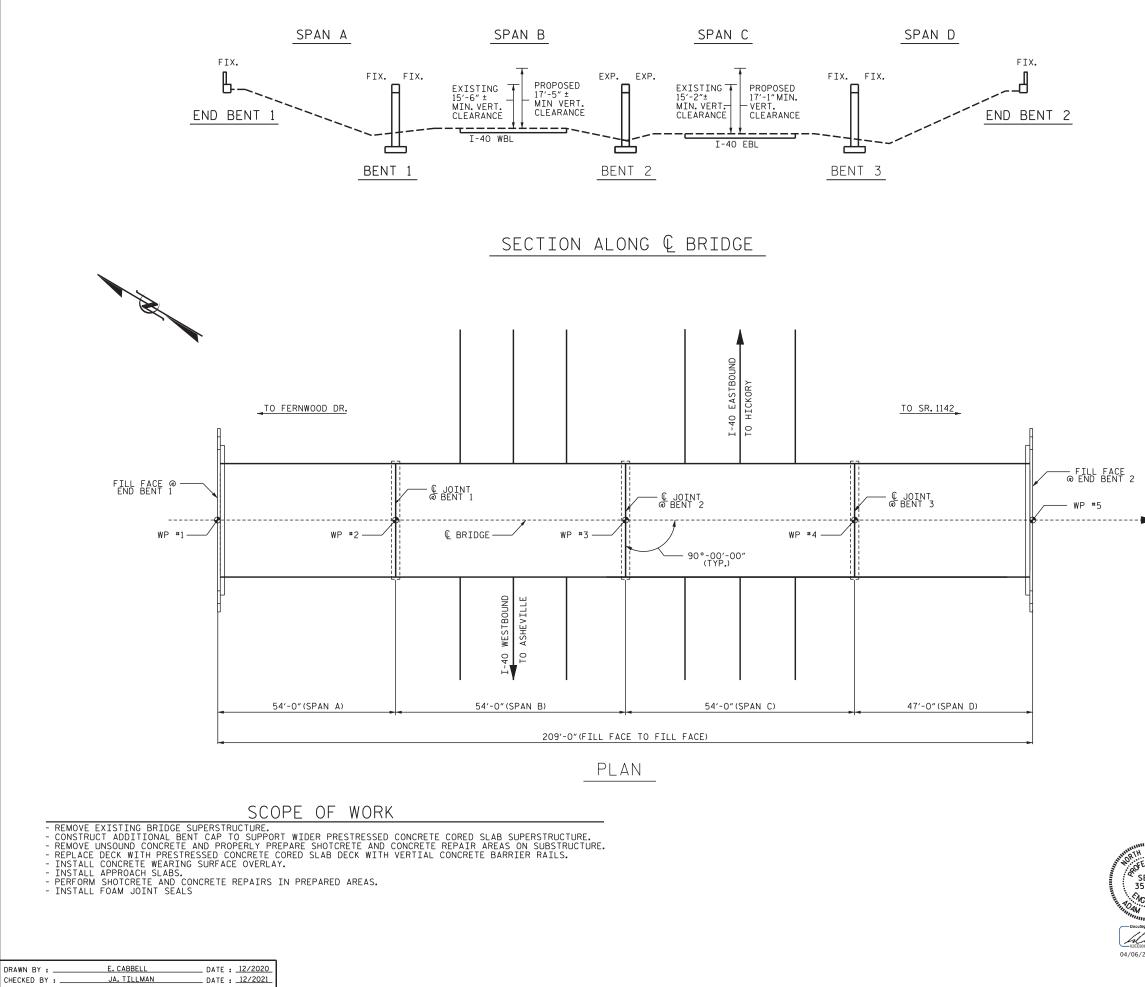
SPECIAL NOTES:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED.CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED.CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.





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NOTES

GENERAL DRAWING INFORMATION IS TAKEN FROM THE ORIGINAL PLANS AND THE ROUTINE INSPECTION REPORT DATED 8/25/2021.

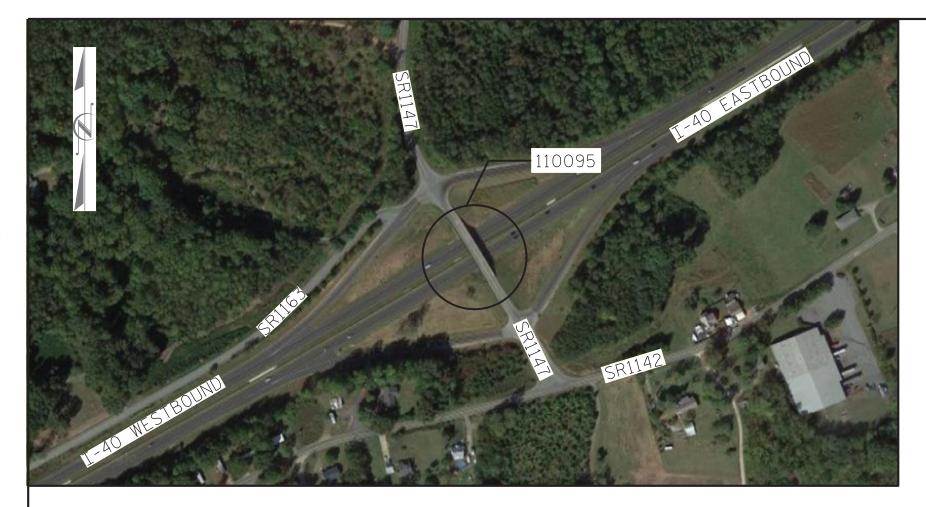
BRIDGE ORIENTATION CONFORMS TO THE ORIGINAL BRIDGE PLANS.

I HEREBY CERTIFY THAT THIS STRUCTURE WAS REHABILITATED ACCORDING TO THESE PLANS OR AS NOTED HEREIN.

DATE

RESIDENT ENGINEER

PROJECT NO. 41665.13A BURKE COUNTY BRIDGE NO. 110095 SHEET 1 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION HURTH CARO CAR RALEIGH GENERAL DRAWING SEAL 031021 ESSIO SEAL 35647 FOR BRDIGE ON SR1417 (CAUSBY RD.) OVER i-40 BETWEEN FERNWOOD DR. AND SR 1142 AGINEER M. WOINES Amber Miller hl B04B5A4=2 04/06/2022 04/06/2022 REVISIONS SHEET NO. DATE: NO. BY: S-01 DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 26



LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION, ONLY. THE CONTRACTOR SHALL CONFIRM, IHROUCH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE COORDINATES LAT: 35.705389

LONG: -81.784528

					— TC)TAL	BILL	OF M	ATER	IAL —						
BRIDGE NO. 110095	INCIDENTAL MILLING	ASPHALT CONCRETE BASE COURSE, TYPE B25.0B	SURFACE COURSE,	ASPHALT BINDER FOR PLANT MIX	ASSBESTOS ASSESSMENT			CLASS A	BRIDGE	REINFORCING STEEL	VERTICAL CONCRETE BARRIER RAIL	ELASTOMERIC BEARINGS	SHOTCRETE REPAIR		PRESTRESSED	OF EXIS
	SQ.YDS.	TONS	TONS	TONS	LUMP SUM	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	LIN.FT.	LUMP SUM	CU.FT.	LUMP SUM	LIN.FT.	LUMP
SUPERSTRUCTURE					LUMP SUM	6,343	6,525.0		LUMP SUM		412.8	LUMP SUM			2,270.1	
END BENT 1								6.6		1,826			1.2			
BENT 1								10.4		1,698			36.0			
BENT 2								10.4		1,698			27.3			
BENT 3								10.4		1,698			52.8			
END BENT 2								6.6		1,826			0			
TOTAL	342.6	60	30	5	LUMP SUM	6,343	6,525.0	44.4	LUMP SUM	8,746	412.8	LUMP SUM	117.3	LUMP SUM	2,270.1	LUMP
		N OF THESE PLANS,														

WILL BE NECESSARY TO PROPERLY COMPLETE THE INTENDED BRIDGE PRESERVATION/REHABILITATION WORK. THE CONTRACTOR SHALL BE PREPARED TO PERFORM SUCH WORK IN A TIMELY MANNER, AS DETERMINED IN THE FIELD WORK SHALL BE CONSIDERED EXTRA WORK AND SHALL BE ADDRESSED AS PER ARTICLE 104-7 OF THE STANDARD SPECIFICATIONS. PROJECT SPECIAL PROVISIONS THAT OUTLINE REQUIREMENTS FOR THESE POTENTIAL ADDITION WORK ITEMS HAVE BEEN PROVIDED IN THE PROJECT DOCUMENTS, BUT NO QUANTITIES HAVE BEEN LISTED. ACTUAL PAY ITEMS, QUANTITIES, AND COSTS WILL BE ESTABLISHED, AS REQUIRED, IF EXTRA WORK IS ENCOUNTERED. UNANTICIPATED ITEMS:

1. CONCRETE REPAIRS SQ.FT.

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DRAWN BY :	E CABBELL	DATE	12/2020
CHECKED BY	:JA.TILLMAN	DATE	: 12/2021

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLANS. FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH BRIDGE, SEE SPECIAL PROVISIONS. FOR PARTIAL REMOVAL OF EXISTING STRUCTURE NO. 110095, SEE SPECIAL PROVISIONS. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS. FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS. FOR FOAM JOINTS, SEE SPECIAL PROVISIONS. FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS. ASSUMED LIVE LOAD= HL-93 OR ALTERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

THE ELEVATION(S) AND CLEARANCE(S) SHOWN ON THE PLANS AT THE POINT(S) OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION(S) ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

ELEVATIONS INDICATED ON THESE PLANS ARE TAKEN FROM THE ORIGINAL BRIDGE PLANS FROM 1956.CONTRACTOR MUST VERIFY THE EXISTING ELEVATIONS AND ANY CORRELATIONS BETWEEN ORIGINAL AND CURRENT DATUM INFORMATION, THE ORIGINAL PLAN ELEVATIONS, AND THE EXISTING CURRENT ELEVATIONS.

PARTIAL REMOVAL OF EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING ONTO THE EXISTING ROADWAY BELOW. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

ANY DAMAGE TO EXISTING REINFORCING STEEL THAT IS TO REMAIN IN PLACE DURING THE CONTRACTOR'S OPERATION SHALL BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST TO THE DEPARTMENT.

FOR PAVEMENT MARKINGS, SEE PAVEMENT MARKING PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

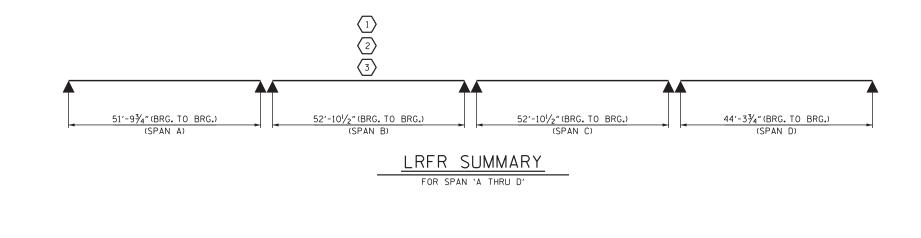
SEE TRAFFIC MANAGEMENT PLANS FOR LANE WIDTHS, SEQUENCING, AND OTHER TRAFFIC CONTROL MEASURES.

NOTES

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

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TIAL DVAL ISTING CTURE		
SUM		
		PROJECT NO. 41665.13A
		BURKE COUNTY
		BRIDGE NO. 110095
SUM		SHEET 2 OF 2
R WORK D.SUCH NAL	I	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING
	SEAL OSIO21 ACINESION CONCERNING CONSERVED CON	FOR BRIDGE ON SR1147 (CAUSBY RD.) OVER I-40 BETWEEN FERNWOOD DR. AND SR 1142
	04/06/2022	REVISIONS SHEET NO.
	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: S-O2 1 3

		LOAD AN	D RES	SIST	ANCE	FAC	CTOR	RAT	ING	(LRF	D) SI	JMMA	RY F	OR F	PRES	TRES	SSED	CON	CRET	E GI	RDEF	rs	
								STRENGTH I LIMIT STATE									SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)
		HL-93(Inv)	NZA	1	1.056		1.75	0.255	1.081	А	EL	25.91	0.530	1.368	А	EL	2.59	1.056	0.254	1.056	В	EL	26.44
DESIGN		HL-93(0pr)	NZA		1.593		1.35	0.255	1.402	А	EL	25.91	0.530	1.773	А	EL	2.59	N/A					
LOAD RATING		HS-20(Inv)	36.000	2	1.321	47.541	1.75	0.255	1.350	А	EL	25.91	0.530	1.635	А	EL	2.59	1.321	0.254	1.321	В	EL	26.44
RATING		HS-20(0pr)	36.000		1.994	71.768	1.35	0.255	1.750	Α	EL	25.91	0.530	2.120	Α	EL	2.59	N/A					
		SNSH	13.500		2.756	37.203	1.40	0.255	3.499	А	EL	25.91	0.530	4.663	А	EL	2.59	2.756	0.254	2.756	В	EL	26.44
		SNGARBS2	20.000		2.147	42.933	1.40	0.255	2.735	А	EL	25.91	0.530	3.378	А	EL	2.59	2.147	0.254	2.147	В	EL	26.44
		SNAGRIS2	22.000		2.074	45.639	1.40	0.255	2.647	А	EL	25.91	0.530	3.160	А	EL	2.59	2.074	0.254	2.074	В	EL	26.44
		SNCOTTS3	27.250		1.374	37.440	1.40	0.255	1.745	А	EL	25.91	0.530	2.334	А	EL	2.59	1.374	0.254	1.374	В	EL	26.44
	S۷	SNAGGRS4	34.925		1.183	41.330	1.40	0.255	1.506	А	EL	25.91	0.530	1.981	А	EL	2.59	1.183	0.254	1.183	В	EL	26.44
		SNS5A	35.550		1.155	41.052	1.40	0.255	1.470	А	EL	25.91	0.530	2.030	А	EL	2.59	1.155	0.254	1.155	В	EL	26.44
		SNS6A	39.950		1.075	42.940	1.40	0.255	1.369	А	EL	25.91	0.530	1.871	А	EL	2.59	1.075	0.254	1.075	В	EL	26.44
LEGAL		SNS7B	42.000		1.024	43.105	1.40	0.255	1.305	А	EL	25.91	0.530	1.863	А	EL	2.59	1.024	0.254	1.024	В	EL	26.44
LOAD		TNAGRIT3	33.000		1.315	43.406	1.40	0.255	1.676	А	EL	25.91	0.530	2.211	А	EL	2.59	1.315	0.254	1.315	В	EL	26.44
RATING		TNT4A	33.075		1.325	43.839	1.40	0.255	1.690	А	EL	25.91	0.530	2.135	Α	EL	2.59	1.325	0.254	1.325	В	EL	26.44
		TNT6A	41.600		1.099	45.718	1.40	0.255	1.403	А	EL	25.91	0.530	2.032	Α	EL	2.59	1.099	0.254	1.099	В	EL	26.44
	ST	TNT7A	42.000		1.113	46.738	1.40	0.255	1.421	А	EL	25.91	0.530	1.909	Α	EL	2.59	1.113	0.254	1.113	В	EL	26.44
	L L	TNT7B	42.000		1.161	48.769	1.40	0.255	1.482	А	EL	25.91	0.530	1.801	А	EL	2.59	1.161	0.254	1.161	В	EL	26.44
		TNAGRIT4	43.000		1.100	47.279	1.40	0.255	1.405	А	EL	25.91	0.530	1.737	А	EL	2.59	1.100	0.254	1.100	В	EL	26.44
		TNAGT5A	45.000		1.029	46.327	1.40	0.255	1.314	А	EL	25.91	0.530	1.756	А	EL	2.59	1.029	0.254	1.029	В	EL	26.44
		TNAGT5B	45.000	3	1.011	45.483	1.40	0.255	1.290	А	EL	25.91	0.530	1.649	Α	EL	2.59	1.011	0.254	1.011	В	EL	26.44



ASSEMBLED BY : E. BAYISSA DATE : 12/2021 CHECKED BY : JA. TILLMAN DATE : 12/2021 DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

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LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ _{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

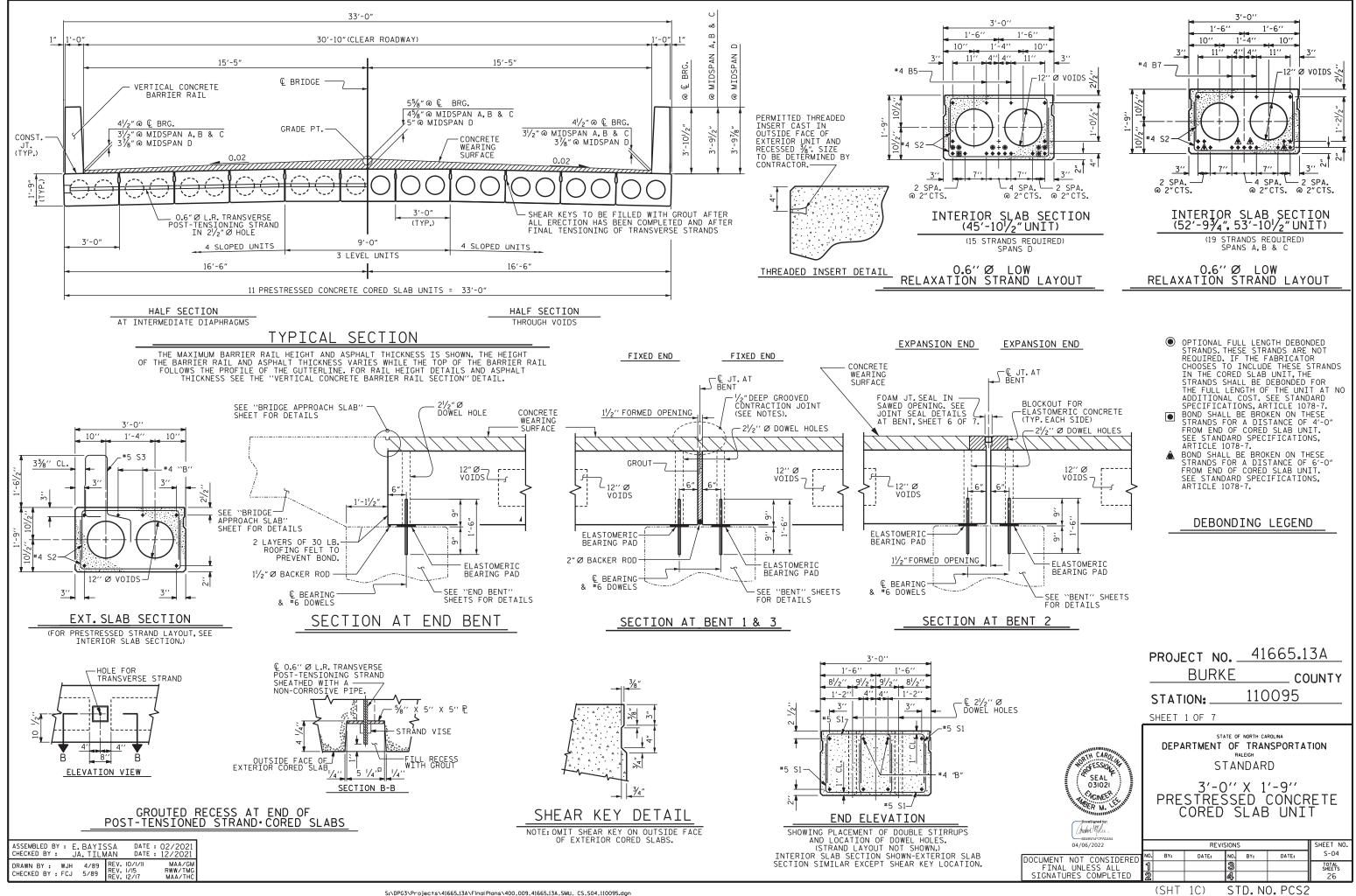
- 1. 2.
- 3.
- 4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
3 LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

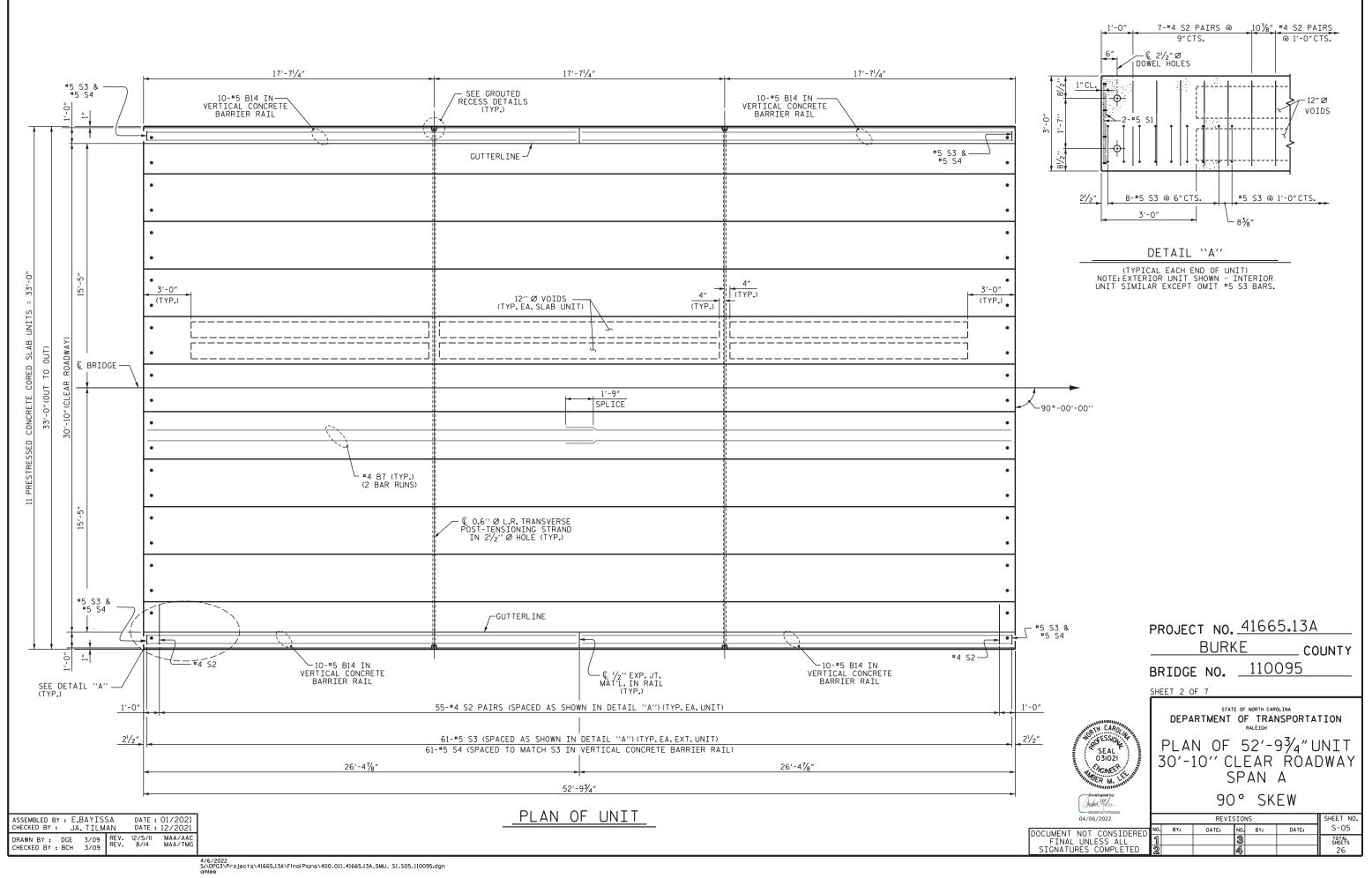
	PROJEC	ET NO. BUR E NO.:	KE	<u>665.1</u> co 2095	<u>3A</u> UNTY
SEAL OSIO21	LR C	RTMENT S FR S ORED	TANDAF UMMA SLAE A TH	NSPORTA RD RY F 3 UNI HRU D	OR T
04/06/2022		REVIS			SHEET NO. S-03
DOCUMENT NOT CONSIDERED	NO. ВҮ: 1	DATE:	NO. BY:	DATE:	TOTAL SHEETS
FINAL UNLESS ALL SIGNATURES COMPLETED	2		4		SHEETS 26

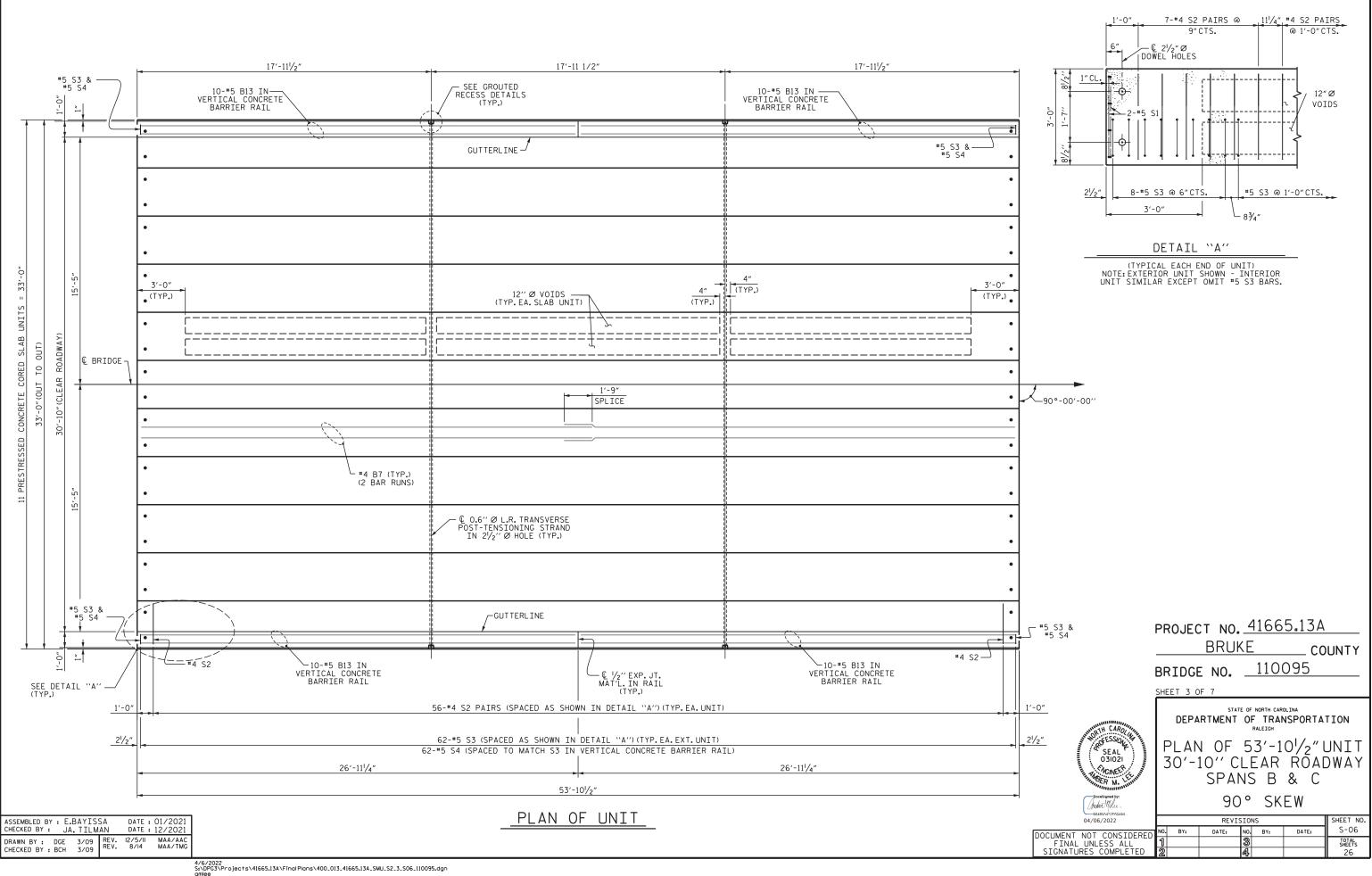
COMMENT NUMBER



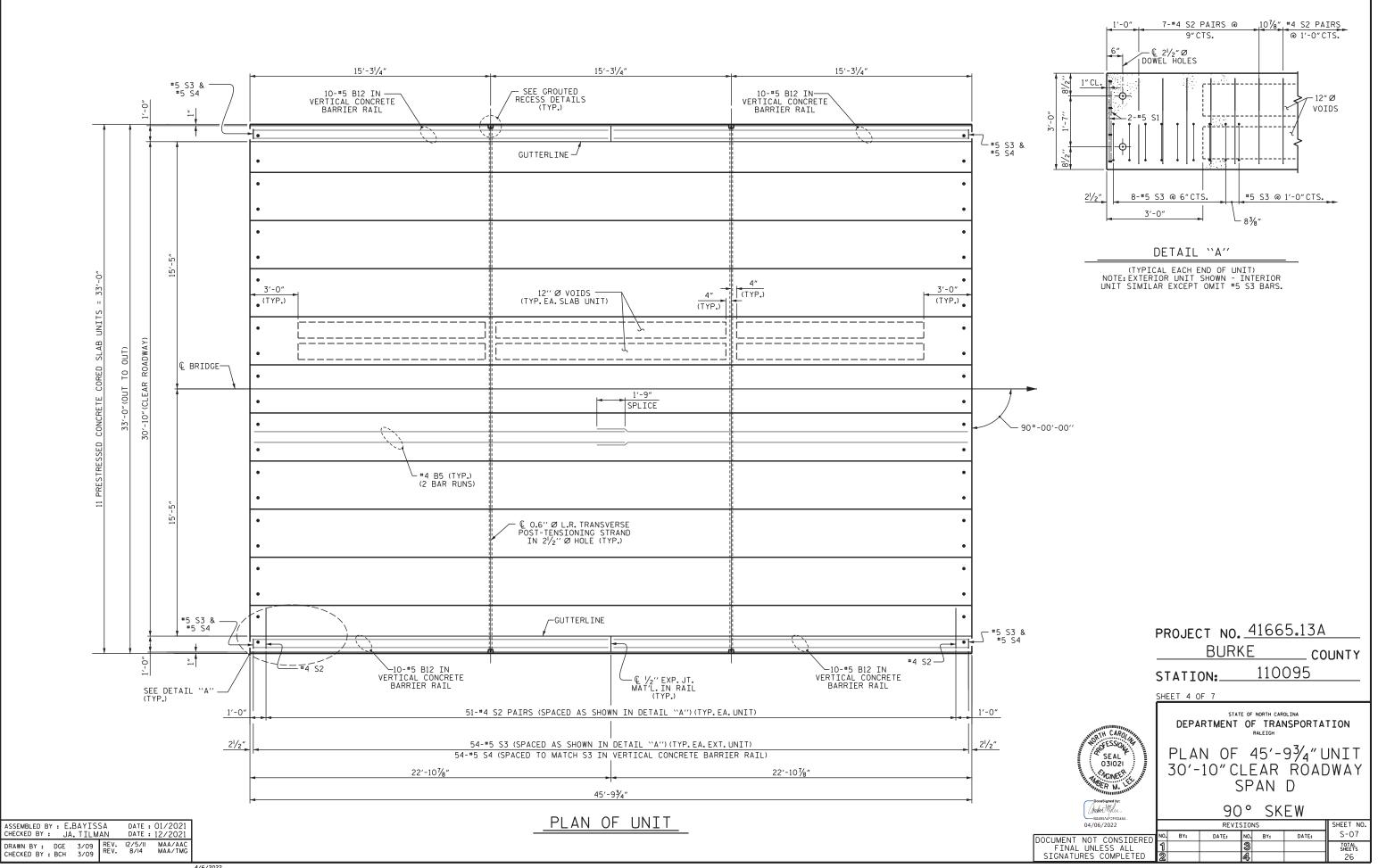
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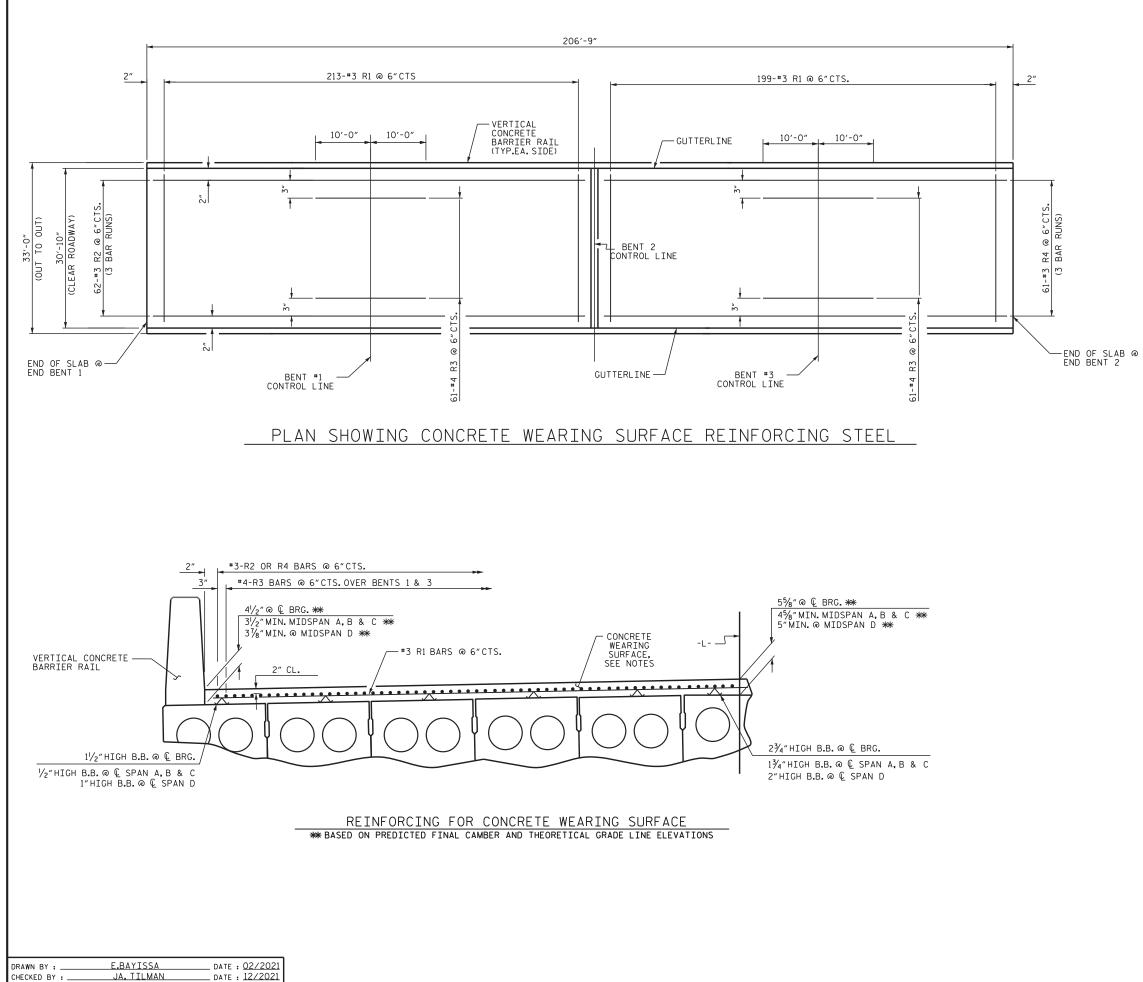




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NOTES

PLACEMENT OF THE CONCRETE WEARING SURFACE SHALL OCCUR AFTER CASTING THE VERTICAL CONCRETE BARRIER RAILS. THE COST OF THE *3 BARS CAST WITH THE CONCRETE WEARING SURFACE SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE WEARING SURFACE.

FOR CONCRETE WEARING SURAFCE, SEE SPECIAL PROVISIONS.

ALL REINFORCING STEEL FOR CONCRETE WEARING SURAFCE SHALL BE EPOXY COATED.

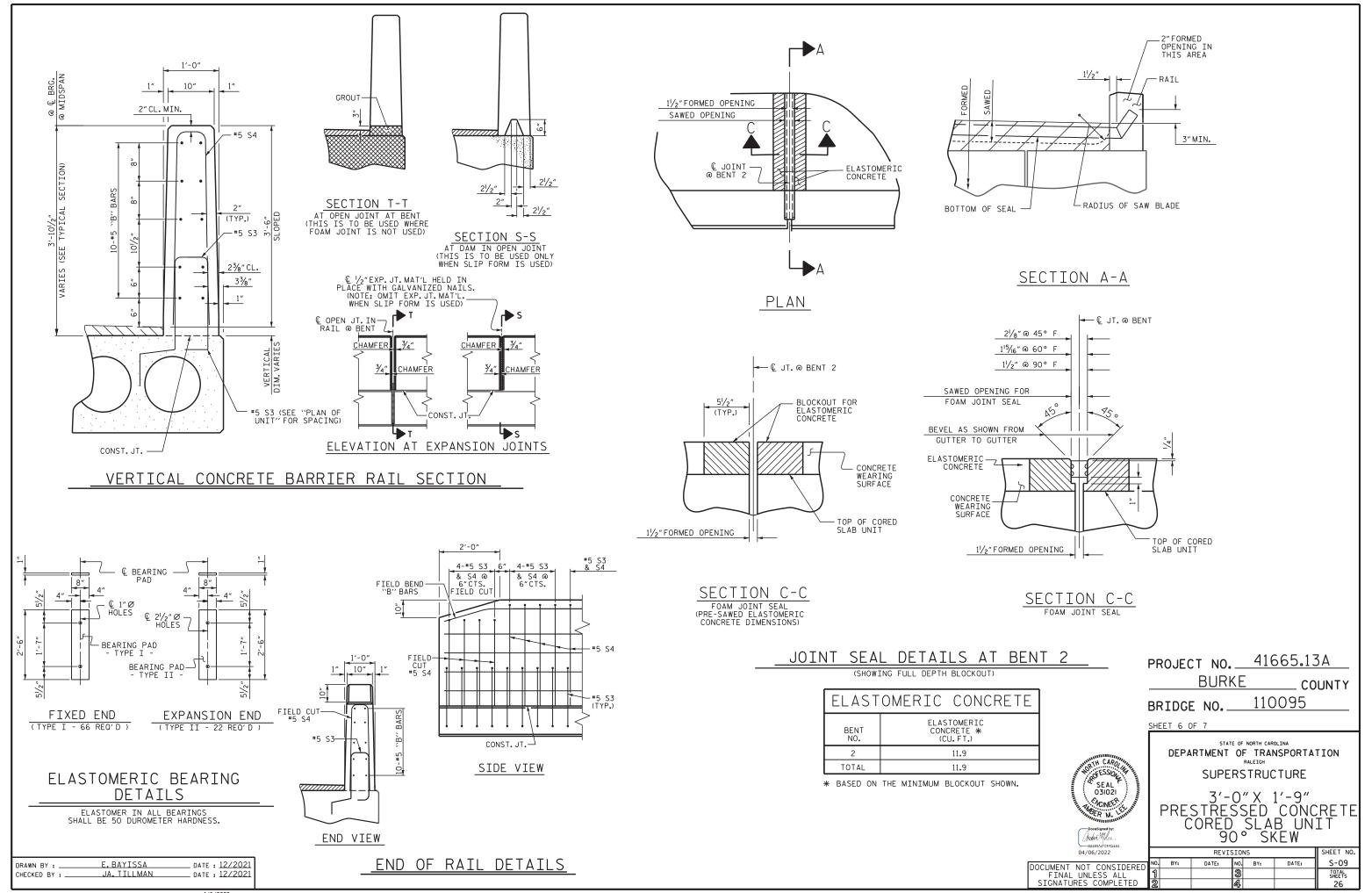
CON	BILL OF MATERIAL FOR CONCRETE WEARING SURFACE								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
*R1	412	#3	STR	30'-6"	4,712				
*R2	186	#3	STR	36'-4"	2,541				
*R3	122	#4	STR	20'-0"	1,630				
*R4	186	#3	STR	34'-0"	2,372				
* EPOXY CO	* EPOXY COATED REINFORCING STEEL LBS. 11,255								
CONCRETE	CONCRETE WEARING SURFACE SO.FT. 6,343								

GROOVING	BRIDGE FLC	ORS
BRIDGE DECK	5,736	SO.FT.
APPROACH SLAB	789	SQ.FT.
TOTAL	6,525	SQ.FT.

SPLICE L	ENGTH CHART
BAR SIZE	EPOXY COATED
#3	1'-3"

	PROJEC	T NO. Buri	< E		<u>3A</u> DUNTY			
	BRIDGE	E NO.:)095				
	SHEET 5 0	F 7						
WINTH CAROLAN	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
SEAL CONCEL	CONCRETE WEARING SURFACE							
Docusigned by: Amplet Molec-								
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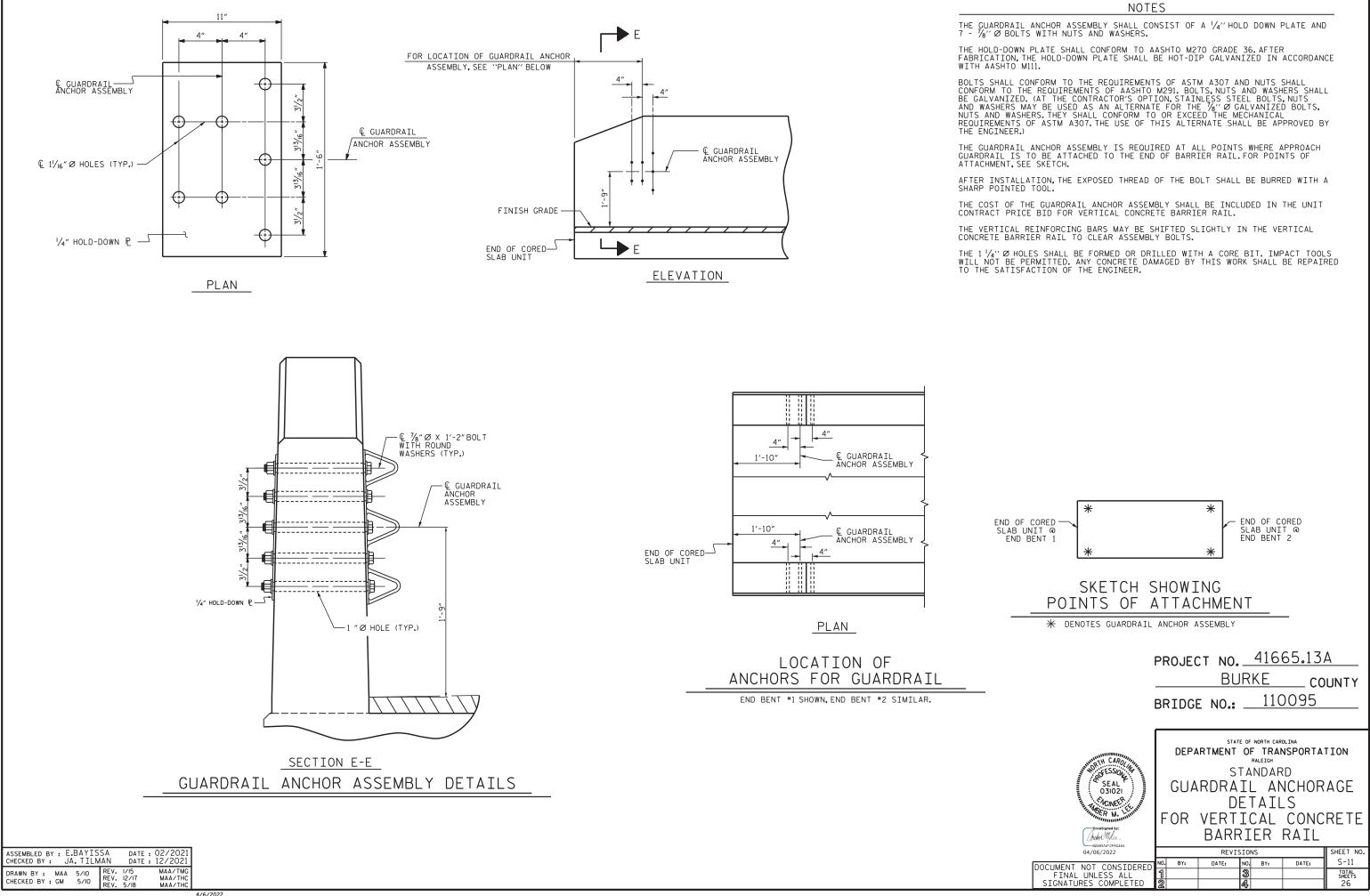
BAR TYPES	BILL OF MAT	ERIAL FOR VERTI	CAL CONCF	RETE	BARR	IER R	AIL	CORED SLAB	S REQUIRED	
7″ 6″		PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	52'-9 ³ / ₄ " UNIT NUMBER	LENGTH TOTAL LENGTH	
<u>S1</u> 1'-9''		9¾″UNIT (SPAN A)							52.8125' 105'-71/2"	ALL PRESTR 270 STRAN
	*B14	40	40	#5	STR	25'-11"	1082	INTERIOR C.S. 9 TOTAL	52.8125′ 475′-3 ³ ⁄4″ 580′-11′⁄4″	REQUIREME SPECIFICA
	* S4	122	122	#5	2	7'-2"	912			ALL REINFO
1 (1) (2) (3) (3) (5) (5) (7) (7) (7) (7) (7) (7) (7) (7) (7) (7	* EPOXY COATED RE	INFORCING STEEL			LBS.	I	1994	CORED SLABS		GRADE 60
	CLASS AA CONCRETE	NCRETE BARRIER RAIL			CU.YDS. LN.FT.		13.9 105.6	SPAN B & C		RECESSES
			<u></u>					EXTERIOR C.S. 4 INTERIOR C.S. 18	53.875' 215'-6" 53.875' 969'-9"	TENSIONIN
₹ <u>2</u> <u>8/4</u> ″ <u>6″</u> <u>7∛4″</u>		ERIAL FOR VERTI PAIR OF EXTERIOR UNITS				IER RA		TOTAL	1185'-3"	THE 21/2"Ø FILLED WI
		2"UNIT (SPANS B & C)	TOTAL NO.	SIZE	TIPE	LENGTH	WEIGHT	CORED SLAB	S REQUIRED	
ALL BAR DIMENSIONS ARE OUT TO OUT	*B13	40	80	#5	STR	26'-6"	2212	45'-9 ³ / ₄ " UNIT NUMBER	LENGTH TOTAL LENGTH	THE BACKER BOND BREAD
								SPAN D EXTERIOR C.S. 2	45 . 8125′ 91′-7 ¹ /2″	WHEN CORED
BILL OF MATERIAL FOR ONE 52'-9¾"CORED SLAB UNIT (SPAN A)	<u>* 54</u>	124	248	#5	2	7'-2"	1854	INTERIOR C.S. 9 TOTAL	45.8125′ 405′-33⁄4″ 503′-11/4″	EMPLOYED SIX WEEKS
EXTERIOR UNIT INTERIOR UNIT	* EPOXY COATED RE CLASS AA CONCRETE	INFORCING STEEL			LBS. CU.YDS.		4066 14 . 2			J TO THE EN PROPOSED
BAR NUMBER SIZE TYPE LENGTH WEIGHT LENGTH WEIGHT B7 4 #4 STR 27'-7" 74 27'-7" 74		NCRETE BARRIER RAIL			LN.FT.		14.2	CONCRETE RELE	ASE STRENGTH	LOCATION
	BTU OF MA	TERIAL FOR VERTI		RETE	BARE	TFR R				ALL REINFO SHALL BE E
S1 8 #5 3 4'-3" 36 4'-3" 36 S2 110 #4 3 5'-4" 392 5'-4" 392	BAR BARS PER	PAIR OF EXTERIOR UNITS	TOTAL NO.			LENGTH		UNIT 45′-9¾″UNITS	PSI 4000	PRESTRESS
* S3 61 *5 1 5'-11" 377	45'-	9¾″UNIT (SPAN D)						45'-9%4" UNITS		ENDS.
	*B12	40	40	#5	STR	22'-5"	936		4900	APPLY EPO
REINFORCING STEEL LBS. 502 502 * EPOXY COATED	* S4	108	108	#5	2	7'-2"	808	53'-10 ¹ /2" UNITS	5000	GROOVED C
REINFORCING STEEL LBS. 377	* EPOXY COATED RE				LBS.	_	1744			825-10(B) 0 BE LOCATE
6000 P.S.I. CONCRETE CU. YDS. 7.5 7.5	CLASS AA CONCRETE				CU.YDS.	•	12.1			JOINTS. ON
0.6" Ø L.R. STRANDS No. 19 19		NCRETE BARRIER RAIL			LN.FT.		91.6			BARRIER R CONTRACTI
	I DEAD LOAD D	EFLECTION AND CA		GRADE	E 270) STRA	NDS			FEET IN LE
BILL OF MATERIAL FOR ONE 53'-10 ¹ / ₂ " CORED SLAB UNIT (SPANS B & C)	52′-9 ¾ ″ UNI	T (SPAN A) 0.6"Ø	L.R.			0.6	″ØL.R.			FLAME CUT ALLOWED.
EXTERIOR UNIT INTERIOR UNIT	CAMBER (SLAB AL	SIRA		REA GOUARE I	INCHES	, (0.217			THE TRANSP
BAR NUMBER SIZE TYPE LENGTH WEIGHT LENGTH WEIGHT	DEFLECTION DUE TO			TIMATE BS.PER	STREN	GTH _	8,600			SHALL BE D STRENGTH (
B7 4 #4 STR 27'-7" 74 27'-7" 74	SUPERIMPOSED DEA	D LOAD TR 78		PLIED F			7.050			``CONCRETE
S1 8 #5 3 4'-3" 36 4'-3" 36 S2 112 #4 3 5'-4" 399 5'-4" 399	FINAL CAMBER			BS. PER	STRAN	5 4	3,950			FOR GROUT
* S3 62 *5 1 5'-11" 383	** INCLUDES FUTUR	EFLECTION AND CA								THE PERMI CONTRACTO
	DEAD LOAD D		< 1'-9"							THE PERMI
REINFORCING STEEL LBS. 509 * EPOXY COATED	53'-10 ¹ /2"UNIT (SPAN B & C) 0.6"@	Ø L.R. AND							SIZED BY IN ACCORD
REINFORCING STEEL LBS. 383	CAMBER (SLAB AL		AND ∕8″ ∳							STAINLESS
6500 P.S.I. CONCRETE CU. YDS. 7.6 7.6		2								THE PERMI IMMEDIATE
0.6" Ø L.R. STRANDS No. 19 19	SUPERIMPOSED DEA	B LOND	́в" ↓							THE COST (
	FINAL CAMBER	RE WEARING SURFACE	"							THE PRICE
BILL OF MATERIAL FOR ONE 45′-9¾″ CORED SLAB UNIT (SPAN D)		EFLECTION AND CA	MRER							POST-TENS
EXTERIOR UNIT INTERIOR UNIT			× 1'-9″							SPECIFICA
BAR NUMBER SIZE TYPE LENGTH WEIGHT LENGTH WEIGHT	45′-9¾″ UNI	T (SPAN D) 0.6"	Ø L.R. RAND							
B5 4 #4 STR 23'-7" 63 23'-7" 63	CAMBER (SLAB ALC		1″ †							
S1 8 #5 3 4'-3" 36 4'-3" 36 S2 102 #4 3 5'-4" 364 5'-4" 364	DEFLECTION DUE TO		3∕8″ ₩							
32 102 "4 3 5-4 364 5-4 364 * S3 54 *5 1 5'-11" 334	SUPERIMPOSED DEAL	, LOAD								
	FINAL CAMBER ** INCLUDES FUTUR		5∕8″ ∔							
REINFORCING STEEL LBS. 463 432	THE THEEDES FUTUR	E MERNING JON ACE								
REINFORCING STEEL LBS. 334										
5000 P.S.I. CONCRETE CU. YDS. 6.6 6.6										
0.6" Ø L.R. STRANDS No. 15 15										
	1									

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NOTES

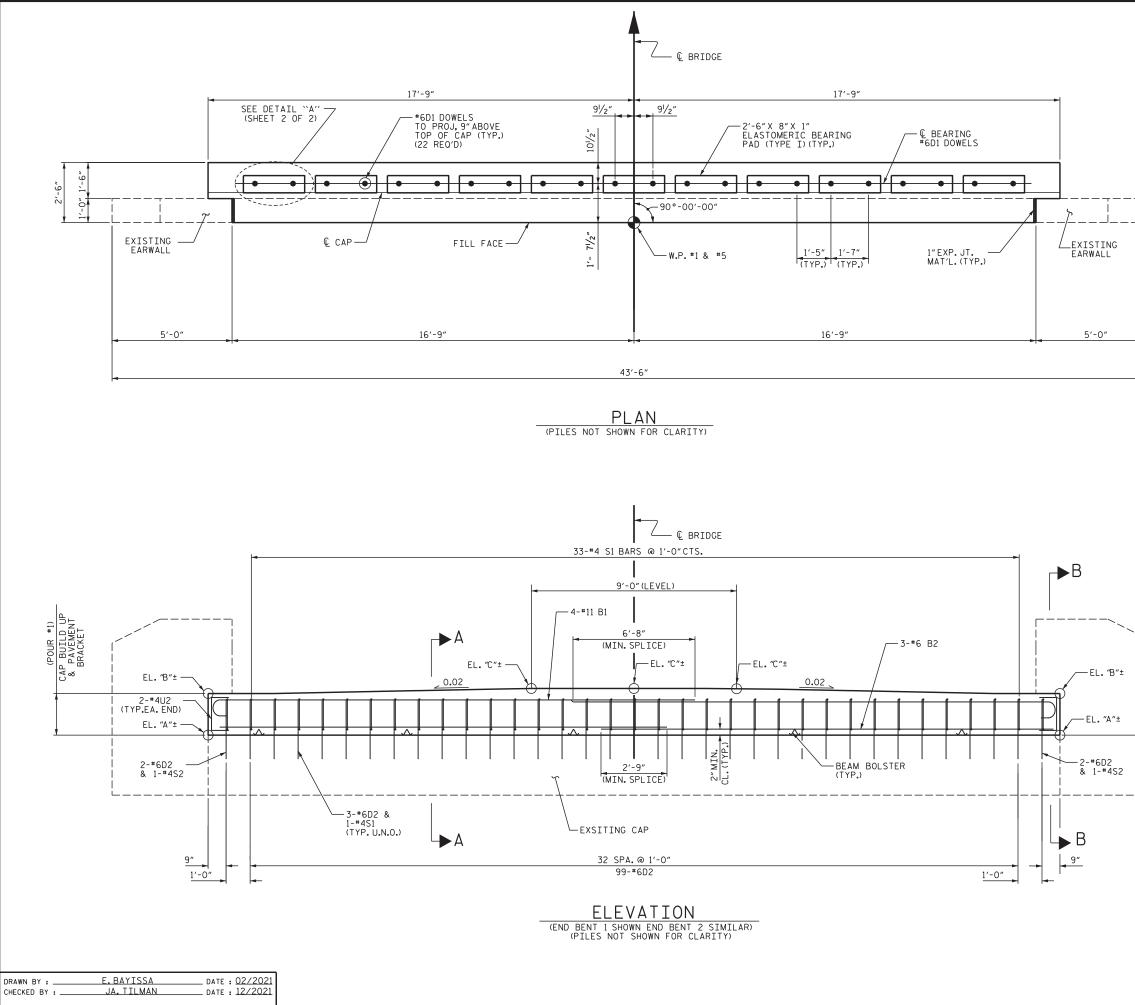
- RESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE NDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING NTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD TIONS.
- FORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR SED CONCRETE CORED SLABS.
- FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE NG OF THE STRANDS.
- DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE TH NON-SHRINK GROUT.
- ER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M AKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
- ED SLABS ARE CAST. AN INTERNAL HOLD-DOWN SYSTEM SHALL BE TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST S PRIOR TO CASTING CORED SLABS. THE CONTRACTOR SHALL SUBMIT NGINEER FOR REVIEW AND COMMENT.DETAILED DRAWINGS OF THE HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
- ORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL EPOXY COATED.
- SING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT
- DXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.
- CONTRACTION JOINTS, $\frac{1}{2}^{\prime\prime}$ IN DEPTH, SHALL BE TOOLED IN ALL FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL ED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO ION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 LENGTH. ENGTH.
- TTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT
- SFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE E RELEASE STRENGTH" TABLE.
- FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- ITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE OR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
- ITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED DANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. S STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.
- ITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR ELY FOLLOWING REMOVAL OF THE FALSEWORK.
- OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN E BID FOR THE PRECAST UNITS.
- SIONING SHALL BE DONE IN ACCORDANCE WITH THE STANDARD TIONS.

	PROJEC	T NO.	416	565 . 13	3A
		BUR	ΚE	CO	UNTY
	BRIDGE	NO.:	11C	095	
	SHEET 7	7 OF 7			
SEAL OSIO2I	DEPA	RTMENT	E OF NORTH CAR OF TRAI RALEIGH RSTRUC	NSPORTA	TION
SEAL O31021	PRES C	3'- STRES ORED 9(SLA		RETE IT
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NOTE

1'-0"

REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

*6D2 DOWELS SHALL BE ADHESIVELY ANCHORED USING AN APPROVED ADHESIVE AND IN ACCORDANCE WITH SUBARTICLE 420-13 OF THE STANDARD SPECIFICATIONS.DOWELS SHALL BE INSTALLED TO THE MINIMUM EMBEDMENT INDICATED ON THE PLANS, UNLESS DEEPER EMBEDMENT IS REQUIRED BY THE ADHESIVE MANUFACTURER.LEVEL ONE FIELD TESTING IS REQUIRED AND THE YIELD LOAD OF THE *6D2 DOWELS IS 26 KIPS.

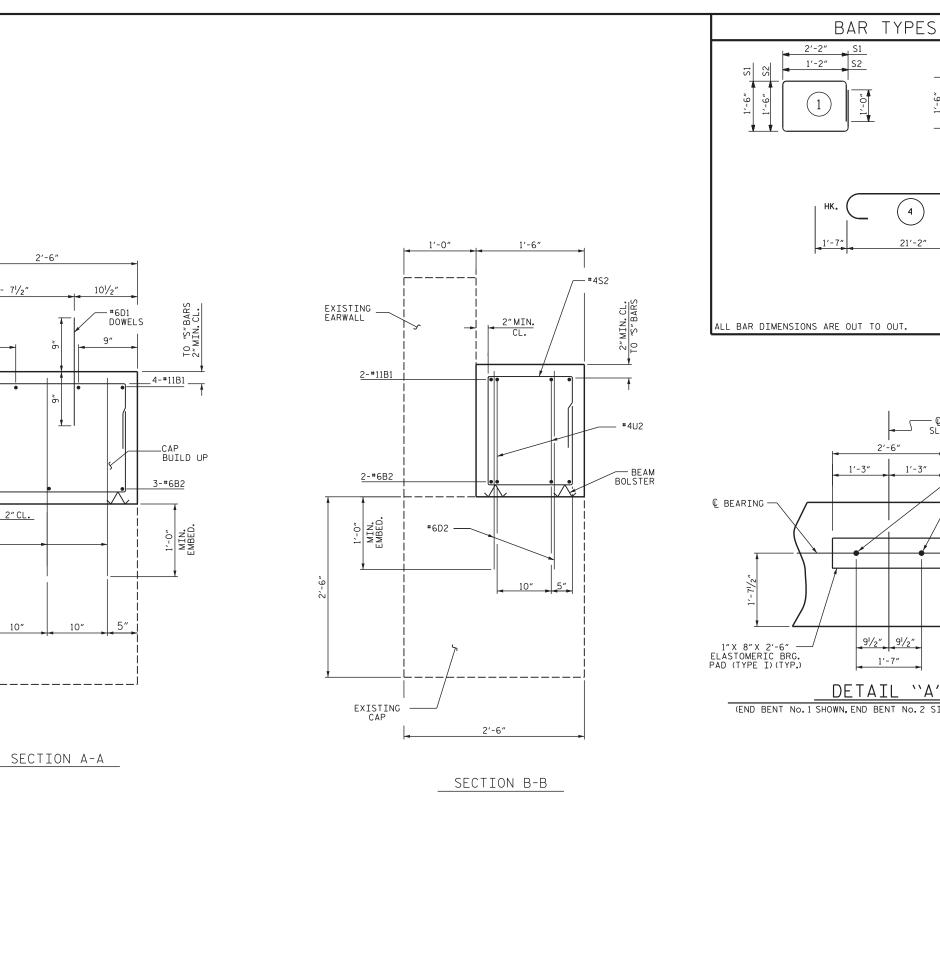
ELEVATIONS INDICATED ON THESE PLANS ARE TAKEN FROM THE ORIGINAL BRIDGE PLANS FROM 1956.CONTRACTOR MUST VERIFY THE EXISTING ELEVATIONS AND ANY CORRELATIONS BETWEEN ORIGINAL AND CURRENT DATUM INFORMATION, THE ORIGINAL PLAN ELEVATIONS, AND THE EXISTING CURRENT ELEVATIONS.

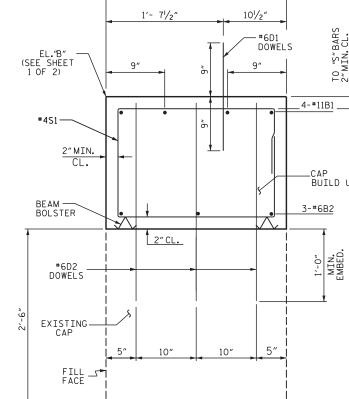
DIMENSIONS AND ELEVATIONS ARE BASED ON AS-BUILT DIMENSIONS AND SCOPING SURVEY INFORMATION. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS PRIOR TO CONSTRUCTION. REINFORCING AND CONCRETE CAP EXTENSIONS SHALL BE ADJUSTED TO MATCH FIELD VERIFIED DIMENSIONS PROVIDED THAT THE OUT-TO-OUT DIMENSIONS REMAIN AS DETAILED.

AFTER REPAIRS TO END BENT CAP, TOP OF END BENT CAP SHALL BE ROUGHEND AND BONDING AGENT SHALL BE PLACED APPRORIATELY PRIOR TO PLACING NEW CONCRETE.

CAP DIMENSIONS							
	END BENT 1	END BENT 2					
"A"	1275.41	1273.92					
"B″	1277.29	1275.75					
"C "	1277.50	1276.02					

2'-6		BURKI	-	CO	<u>3A</u> UNTY		
	BRIDG	_ NU	110	/035			
	SHEET 1 O	F 2					
AND REAL CAROLINE	DEPA				TION		
SEAL A SEAL A MONECAL	SUBSTRUCTURE SUBSTRUCTURE END BENT 1 & 2						
Docusigned by: Andre Male -							
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	Consider the card of the card	BRIDGE SHEET 1 OF DEPA	BURKI BRIDGE NO	BURKE BRIDGE NO. 11C SHEET 1 OF 2 STATE OF NORTH CAR STATE OF NORTH CAR DEPARTMENT OF TRAI RALEIGH SUBSTRUC END BENT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL DOCUMENT NOT CONSIDERED	BURKE CO BRIDGE NO. 110095 SHEET 1 OF 2 SHEET 1 OF 2 SHEE		





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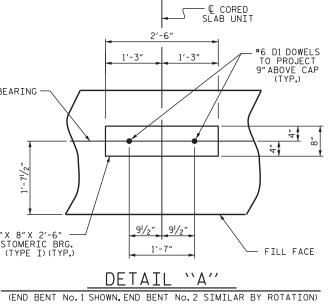
"O-,

1'-4" U2 (2)1'-6"



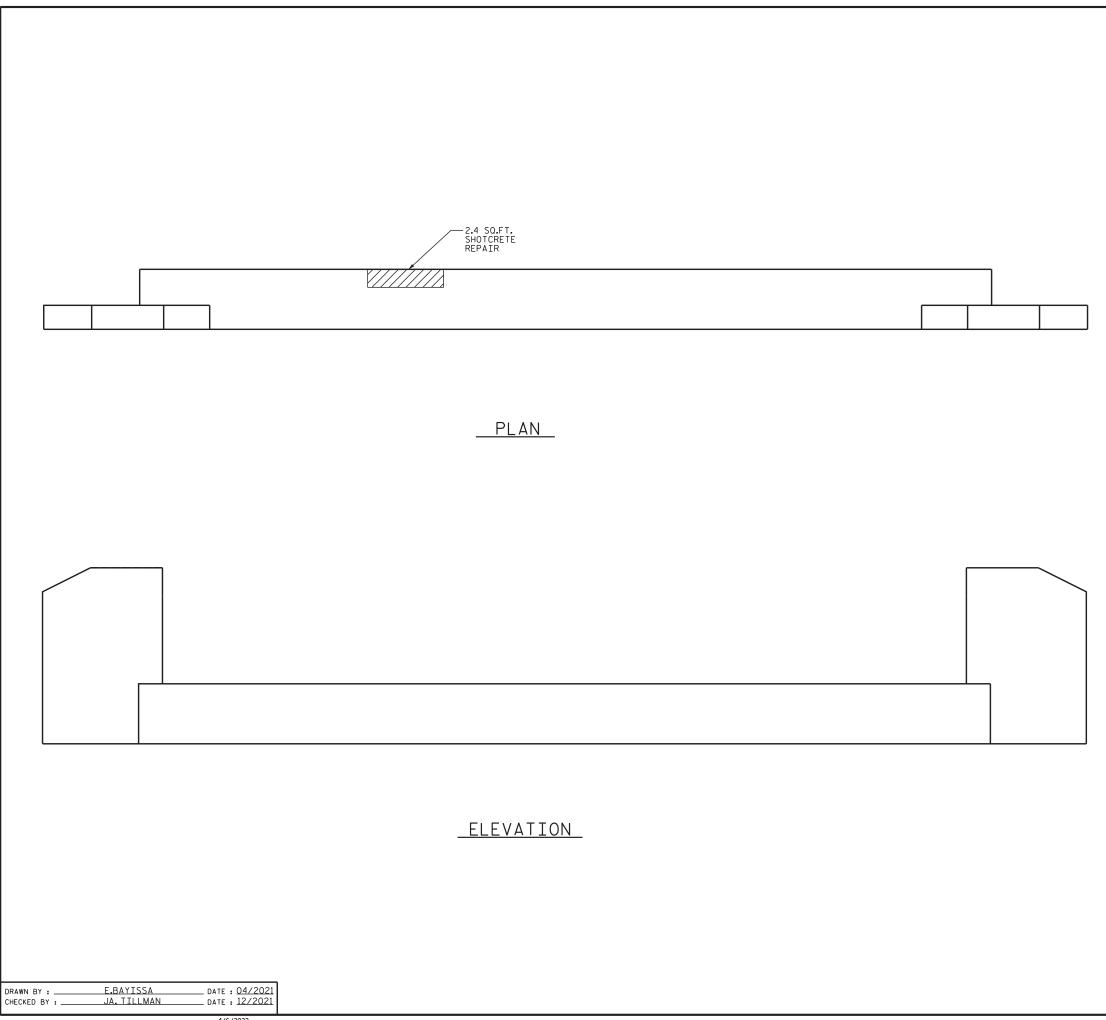
2'-6"

1'-7"



BILL OF MATERIAL								
	Т 1							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
B1	8	#11	4	22'-9"	967			
B2	6	#6	STR.	19'-0"	171			
D1	22	#6	STR.	1'-6"	50			
D2	103	#6	STR.	2'-8"	413			
S1	33	#4	1	8'-10"	195			
S2	4	#4	1	6'-10"	18			
U2	4	#4	2	4'-4"	12			
R	EINFORG	CING S	STEEL	LBS.	1826			
CLASS	"A" CON	CRETE	BREAK	COWN :				
	(POUR 1) CAP		С.Ү.	6.6			
TOTAL	CLASS	"A" CO	NCRET	: C.Y.	6.6			
TOTAL		″a″ co END	NCRETE BEN		6.6			
TOTAL BAR					6.6 WEIGHT			
		END	BEN	Т 2				
BAR	NO.	END SIZE	BEN TYPE	T 2 LENGTH	WEIGHT			
BAR B1	NO. 8	END SIZE #11	BEN Type 4	T 2 LENGTH 22'-9"	WEIGHT 967			
BAR B1	NO. 8	END SIZE #11	BEN TYPE 4 STR. STR.	T 2 LENGTH 22'-9" 19'-0" 1'-6"	WEIGHT 967			
BAR B1 B2	NO. 8 6	END SIZE #11 #6	BEN TYPE 4 STR.	T 2 LENGTH 22'-9" 19'-0"	WEIGHT 967 171			
BAR B1 B2 D1 D2	NO. 8 6 22 103	END SIZE #11 #6 #6 #6	BEN TYPE 4 STR. STR.	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8"	WEIGHT 967 171 50 413			
BAR B1 B2 D1 D2 S1	NO. 8 6 22 103 33	END SIZE #11 #6 #6 #6 #4	BEN TYPE 4 STR. STR. STR. 1	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10"	WEIGHT 967 171 50 413 195			
BAR B1 B2 D1 D2	NO. 8 6 22 103	END SIZE #11 #6 #6 #6	BEN TYPE 4 STR. STR. STR.	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8"	WEIGHT 967 171 50 413			
BAR B1 B2 D1 D2 S1	NO. 8 6 22 103 33	END SIZE *11 *6 *6 *4 *4	BEN TYPE 4 STR. STR. STR. 1 1	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10"	WEIGHT 967 171 50 413 195 18			
BAR B1 B2 D1 D2 S1	NO. 8 6 22 103 33	END SIZE #11 #6 #6 #6 #4	BEN TYPE 4 STR. STR. STR. 1	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10"	WEIGHT 967 171 50 413 195			
BAR B1 B2 D1 D2 S1 S2 U2	NO. 8 6 22 103 33 4 4 4	END SIZE *11 *6 *6 *6 *4 *4 *4 *4	BEN TYPE 4 STR. STR. 1 1 1 2	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4"	WEIGHT 967 171 50 413 195 18 12			
BAR B1 B2 D1 D2 S1 S2 U2	NO. 8 6 22 103 33 4	END SIZE *11 *6 *6 *6 *4 *4 *4 *4	BEN TYPE 4 STR. STR. 1 1 1 2	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10"	WEIGHT 967 171 50 413 195 18			
BAR B1 B2 D1 D2 S1 S2 U2 R	NO. 8 6 22 103 33 4 4 E INFORC	END SIZE #11 #6 #6 #4 #4 #4 #4	BEN TYPE 4 STR. STR. 1 1 1 2 STEEL	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4" LBS.	WEIGHT 967 171 50 413 195 18 12			
BAR B1 B2 D1 D2 S1 S2 U2 U2 R CLASS	NO. 8 6 22 103 33 4 4 E INFORC "A" CON	END SIZE *11 *6 *6 *4 *4 *4 CING S CRETE	BEN TYPE 4 STR. STR. 1 1 1 2 STEEL	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4" LBS. COOWN :	WEICHT 967 171 50 413 195 18 12 1826			
BAR B1 B2 D1 D2 S1 S2 U2 U2 R CLASS	NO. 8 6 22 103 33 4 4 E INFORC	END SIZE *11 *6 *6 *4 *4 *4 CING S CRETE	BEN TYPE 4 STR. STR. 1 1 1 2 STEEL	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4" LBS.	WEIGHT 967 171 50 413 195 18 12			
BAR B1 B2 D1 D2 S1 S2 U2 U2 R CLASS	NO. 8 6 22 103 33 4 4 E INFORC "A" CON	END SIZE *11 *6 *6 *4 *4 *4 CING S CRETE	BEN TYPE 4 STR. STR. 1 1 1 2 STEEL	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4" LBS. COOWN :	WEICHT 967 171 50 413 195 18 12 1826			
BAR B1 B2 D1 D2 S1 S2 U2 U2 R1 CLASS	NO. 8 6 22 103 33 4 4 E INFORC "A" CON	END SIZE #11 #6 #6 #4 #4 #4 CING S CRETE CAP	BEN TYPE 4 STR. STR. 1 1 1 2 STEEL BREAF	T 2 LENGTH 22'-9" 19'-0" 1'-6" 2'-8" 8'-10" 6'-10" 4'-4" LBS. C.Y.	WEICHT 967 171 50 413 195 18 12 1826			

	PROJEC		ΚE	65.13 co 095	<u>3A</u> UNTY
SEAL SEAL SEAL SEAL SEAL SEAL SEAL SEAL		RTMENT SUBS END B	RALEIGH	NSPORTA TURE 1 & 2	
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AS-BUILT REPAIR	QUAN	NTITY	ή ΤΑΙ	BLE
END BENT 1		QUANTI	LTIES	
END DENT I	EST	IMATE	AC	TUAL
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.
CAP	2.4	1.2		
CURTAIN WALL	0.0	0.0		
WING WALL	0.0	0.0		
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.
CAP	0.0	0.0		
CURTAIN WALL	0.0	0.0		
WING WALL	0.0	0.0		
EPOXY RESIN INJECTION		LIN.FT.	LIN	I.FT.
CAP		0.0		
EAR WALL		0.0		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

NOTES

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE AS-BUILT REPAIR QUANTITY TABLE.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

FOR CAP AND COLUMN REPAIR DETAILS, SEE SHEET S-25.

FOR ADDITIONAL END BENT 1 PLANS, SEE SHEETS S-12 THRU S-13.



SHOTCRETE REPAIR AREA



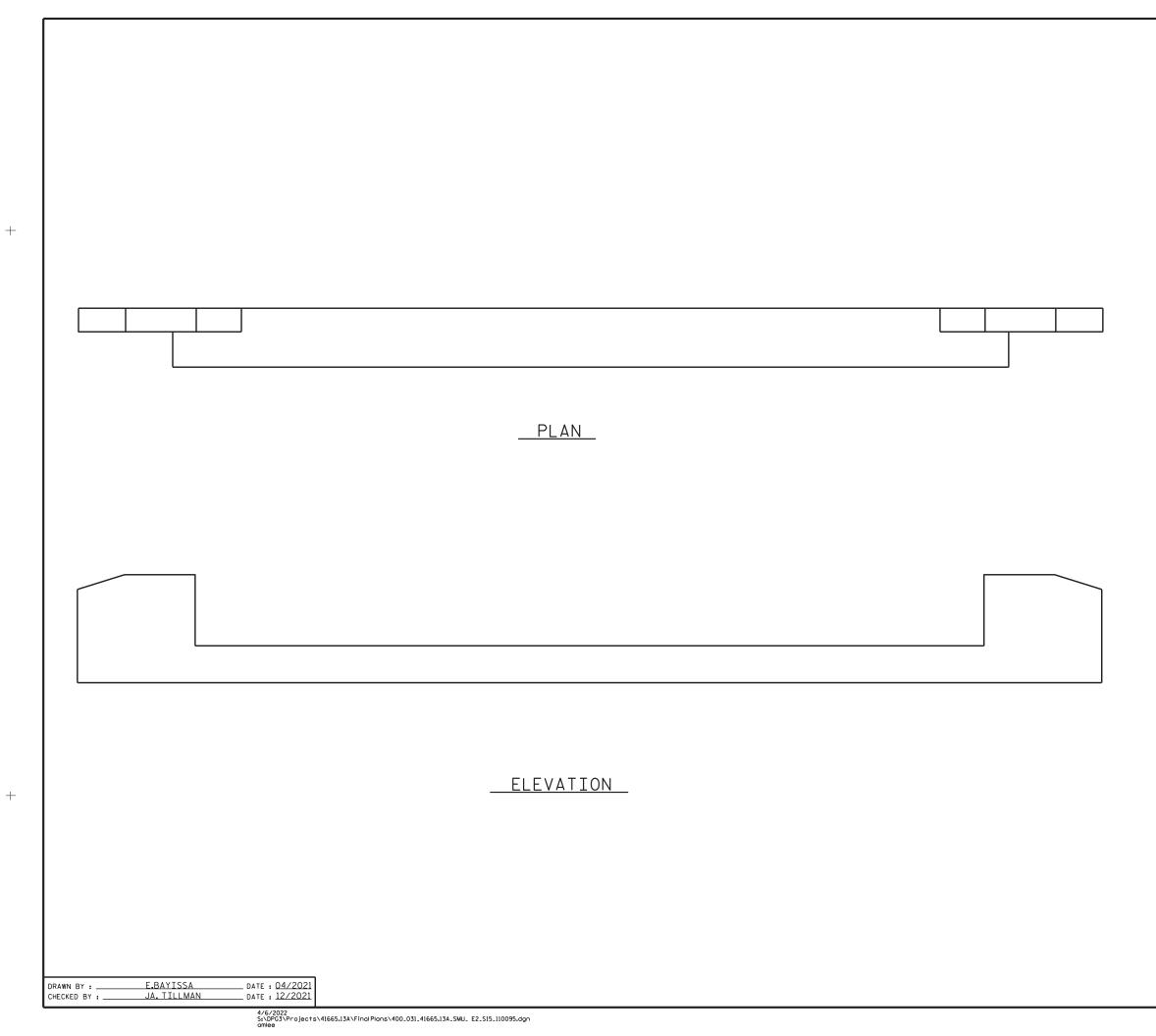
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CONCRETE REPAIR AREA



	PROJECT NO	41665	5.13A
	BURKE	<u>=</u>	COUNTY
	BRIDGE NO.: .	11009	15
RTH CAROLINA	STATE OF	F NORTH CAROLINA F TRANSPO RALEIGH	RTATION
SEAL 031021	SUBST END	RUCTU BENT	RE 1

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SIGNATURES COMPLETED	2			4			26



AS-BUILT REPAIR	QUAN	VTITY	ή ΤΑ	BLE
END BENT 2		QUANTI		
LIND DEINT Z	EST	IMATE	AC	TUAL
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.
CAP	0.0	0.0		
CURTAIN WALL	0.0	0.0		
WING WALL	0.0	0.0		
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.
CAP	0.0	0.0		
CURTAIN WALL	0.0	0.0		
WING WALL	0.0	0.0		
EPOXY RESIN INJECTION		LIN.FT.	LIN.FT.	
CAP		0.0		
EAR WALL		0.0		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

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FOR CAP AND COLUMN REPAIR DETAILS, SEE SHEET S-25.

FOR ADDITIONAL END BENT PLANS, SEE SHEETS S-12 THRU S-13.

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SHOTCRETE REPAIR AREA

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CONCRETE REPAIR AREA

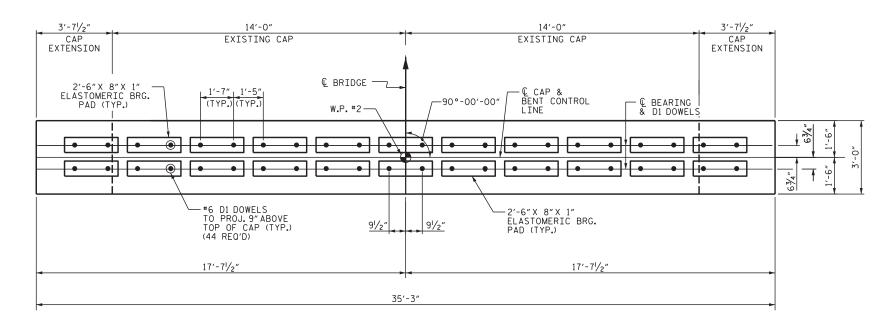
ERI - EPOXY RESIN INJECTION

PROJECT NO. 41665.13A BURKE COUNTY BRIDGE NO.: 110095

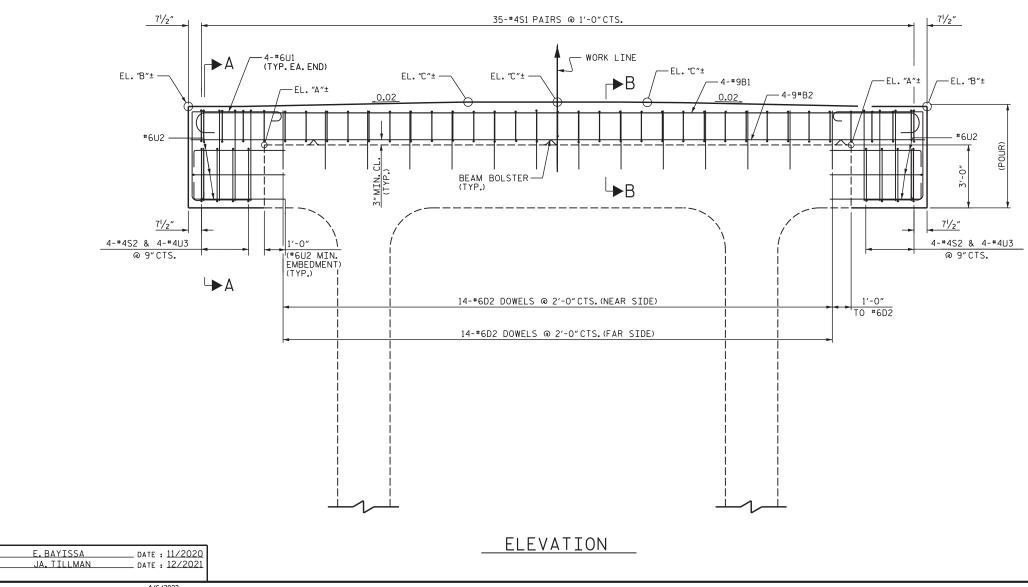
> STATE OF NORTH CAROLINA

SUBSTRUCTURE END BENT 2

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NOTE:

REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

*6D2 & *6U2 DOWELS SHALL BE ADHESIVELY ANCHORED USING AN APPROVED ADHESIVE AND IN ACCORDANCE WITH SUBARTICLE 420-13 OF THE STANDARD SPECIFICATIONS. DOWELS SHALL BE INSTALLED TO THE MINIMUM EMBEDMENT INDICATED ON THE PLANS, UNLESS DEEPER EMBEDMENT IS REOUIRED BY THE ADHESIVE MANUFACTURER.LEVEL ONE FIELD TESTING IS REOUIRED AND THE YIELD LOAD OF THE *6D2 & *6U2 DOWELS IS 26 KIPS.

DIMENSIONS AND ELEVATIONS ARE BASED ON AS-BUILT DIMENSIONS AND SCOPING SURVEY INFORMATION. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS PRIOR TO CONSTRUCTION. REINFORCING AND CONCRETE CAP EXTENSIONS SHALL BE ADJUSTED TO MATCH FIELD VERIFIED DIMENSIONS PROVIDED THAT THE OUT-TO-OUT DIMENSIONS REMAIN AS DETAILED.

ELEVATIONS INDICATED ON THESE PLANS ARE TAKEN FROM THE ORIGINAL BRIDGE PLANS FROM 1956. CONTRACTOR MUST VERIFY THE EXISTING ELEVATIONS AND ANY CORRELATIONS BETWEEN ORIGINAL AND CURRENT DATUM INFORMATION, THE ORIGINAL PLAN ELEVATIONS, AND THE EXISTING CURRENT ELEVATIONS.

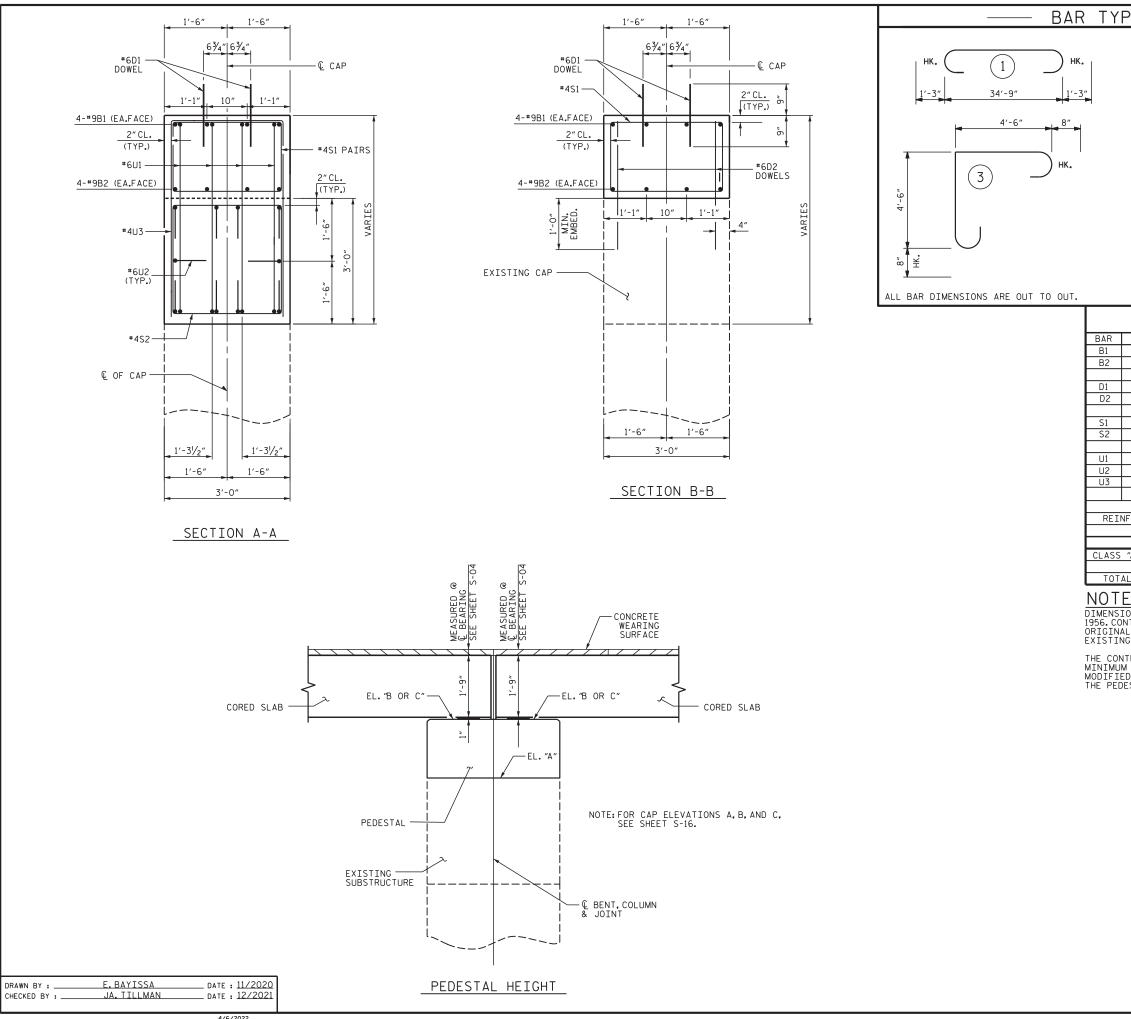
AFTER REPAIRS TO BENT CAP, TOP AND ENDS OF BENT CAP SHALL BE ROUGHEND AND BONDING AGENT SHALL BE PLACED APPROPRIATELY PRIOR TO PLACING NEW CONCRETE.

USE ELASTOMERIC BEARING PAD, TYPE I AT BENTS 1 & 3.

USE ELASTOMERIC BEARING PAD, TYPE II AT BENT 2.

CAP ELEVATIONS							
	BENT 1	BENT 2	BENT 3				
"A"	1275.60	1275.39	1274.76				
<i>"</i> B″	1277.44	1277.23	1276.59				
"C "	1277.70	1277.49	1276.87				

	PROJEC	BURI	< <u>E</u>	-		<u>3A</u> UNTY
	SHEET 1 O	2				
SEAL OSIO21	DEPA	RTMENT SUBS	OF T	RUC		TION
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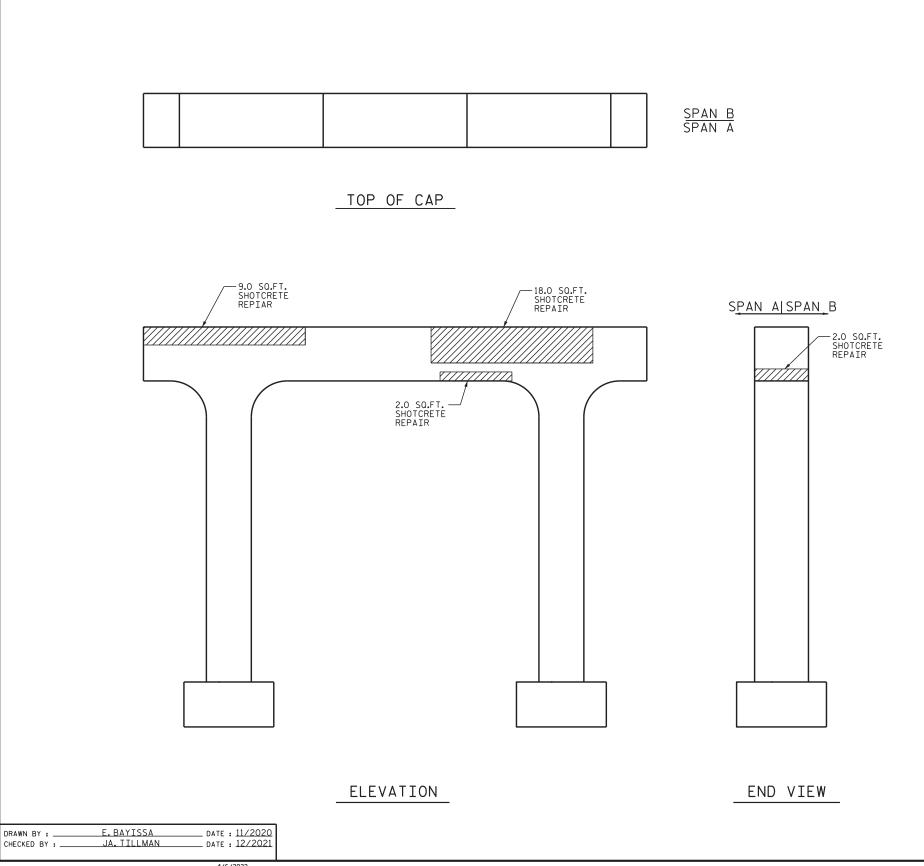
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		4	1'-4"				FOF	r be	NT 1	
					BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
-	— i—			<u> </u>	B1	4	#9	1	37'-3"	507
1′-0″		(2)		B2	4	#9	STR	34'-9″	473
÷.			<u> </u>		D1	44	#6	STR.	1'-6"	99
-	_'				D2	28	#6	STR.	2'-8"	112
		2'	'-8″ S2)		20				
	S2	-	<u>'-8" S2</u>		S1	70	#4	5	5′-6″	257
	-			-	S2	8	#4	4	11'-8"	62
	2'-8"		4	, 0-,1	1.11		#4	7	10'-4"	FF
	ý.			ì.	U1 U2	8 20	#4 #4	3	10°-4″ 5′-4″	55 71
	<u> </u>			-	U2 U3	8	#4	5	11'-8"	62
r		2'	-8″	U3		•	·	·		
SI					REI	NFORCI	NG STE	EL	LBS.	1698
. † .	†		\sim							
1-5" 4'-6"		(!	5)			//// CON				
4 م	·	\sim			CLASS "A" CONCRETE BREAKDOWN : CAP C.Y.					10.4
<u> </u>	<u>¥</u> _		I		тот	AL CLAS	· · · · · · · · · · · · · · · · · · ·	CONCRE		10.4
	FOF	r be	NT 2				FC	R B	ENT 3	
N0.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
4	#9	1	37'-3"	507	B1	4	#9	1	37'-3"	507
4	#9	STR	34'-9"	473	B2	4	#9	STR	34'-9"	473
		CTD	11 6 "					CTD	11.0"	
44	#6 #6	STR. STR.	1'-6" 2'-8"	99 112	D1 D2	44	#6 #6	STR. STR.	1'-6" 2'-8"	99 112
28	0	311.	2 -0	112	02	28	0	3114.	2 -0	112
70	#4	5	5′-6″	257	S1	70	#4	5	5′-6″	257
8	#4	4	11'-8"	62	S2	8	#4	4	11'-8"	62
								_		
8	#4	3	10'-4"	55	U1	8	#4	3	10'-4"	55
20 8	#4 #4	2	5'-4" 11'-8"	71 62	U2 U3	20 8	#4 #4	2	5'-4" 11'-8"	71 62
0			11 0	02	05	0		5	11 0	02
						I				
FORCING STEEL LBS. 1698 REINFORCING STEEL LBS.					1698					
		0057	(DOW):		01.107			005	0.000	
"A" CON	<u>NCRETE</u> CAP	BREAK	(DOWN :	10.4	CLASS	"A" CON	<u>NCRETE</u> CAP	RKEAF	(DOWN :	10.4
AL CLAS		CONCRE	C.Y. ETE: C.Y.	10.4	тот	AL CLAS		CONCRE	C.Y. ETE: C.Y.	10.4
	55 A	SUNCIL			101	AL ULA.	55 M	SUNCIL		
Ε										

DIMENSIONS AND ELEVATIONS SHOWN ARE TAKEN FROM THE ORIGINAL BRIDGE PLANS FROM 1956. CONTRACTOR MUST VERIFY THE EXISTING ELEVATIONS AND CORRELATIONS BETWEEN ORIGINAL AND CURRENT DATUM INFORMATION, THE ORIGINAL PLAN ELEVATIONS, AND THE EXISTING CURRENT ELEVATIONS.

THE CONTRACTOR SHALL MAKE ADJUSTMENTS TO PEDESTAL EL. \mathcal{B}'' AND \mathcal{C}'' TO MAINTAIN MINIMUM $3^{l}\!/_{2}''$ CONCRETE WEARING SURFACE @ \mathbb{Q} BEARING. IF ANY ELEVATIONS ARE MODIFIED, THE CONTRACTOR IS RESPONSIBLE FOR ADJUSTING THE REINFORCING TO FIT THE PEDESTAL HEIGHT.

	PROJEC	CT NO.	416	65.13	3A	
		BURKE co				
	BRIDGE	RIDGE NO. 110095				
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SEAL OSIO21 December dr. Baland Dr. Baland Dr. Baland Dr.	DEPA	RTMENT SUBS	e of north car OF TRAN RALEIGH TRUC 1, 2	NSPORTA TURE	TION	
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AS-BUILT REPAIR QUANTITY TABLE							
BENT 1 SPAN A FACE		QUANT					
DENT I SI AN A I AGE	ESTI	MATE	ACT	UAL			
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.			
CAP	31.0	16.0					
COLUMN	0.0	0.0					
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.			
CAP	0.0	0.0					
COLUMN	0.0	0.0					
EPOXY RESIN INJECTIO	LIN.FT.	LIN.FT.					
CAP	0.0						
COLUMN		0.0					

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1" BEHIND REBAR AND MINIMUM 2" CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

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FOR ADDITIONAL BENT 1 PLANS, SEE SHEETS S-16 THRU S-17.



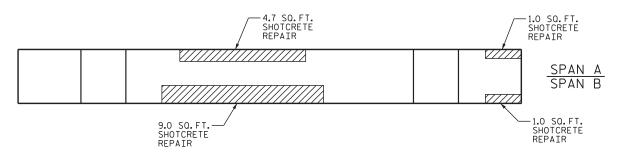
SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

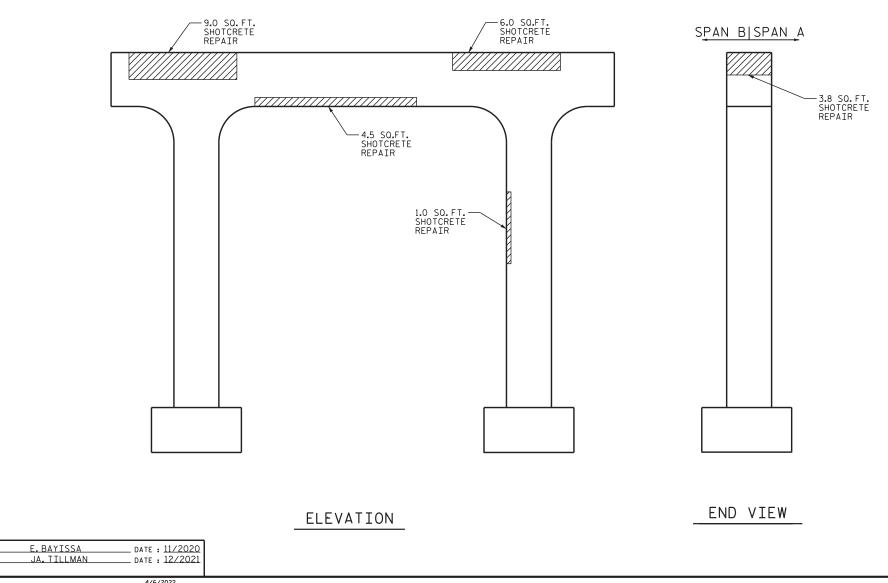


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of ESSION		RTMENT	RALEIGH	NSPORTA		
SEAL SIDE	SUBSTRUCTURE REPAIR BENT 1 SPAN A FACE					
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AS-BUILT REPAIR QUANTITY TABLE							
BENT 1 SPAN B FACE	ГСТТ	QUANTITIES ESTIMATE ACTUAL					
	ESII		ACT				
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.			
CAP	39.0	19.5					
COLUMN	1.0	0.5					
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.			
CAP	0.0	0.0					
COLUMN	0.0	0.0					
EPOXY RESIN INJECTIC	LIN.FT.	LIN	.FT.				
САР		0.0					
COLUMN		0.0					

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

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SHOTCRETE REPAIR AREA

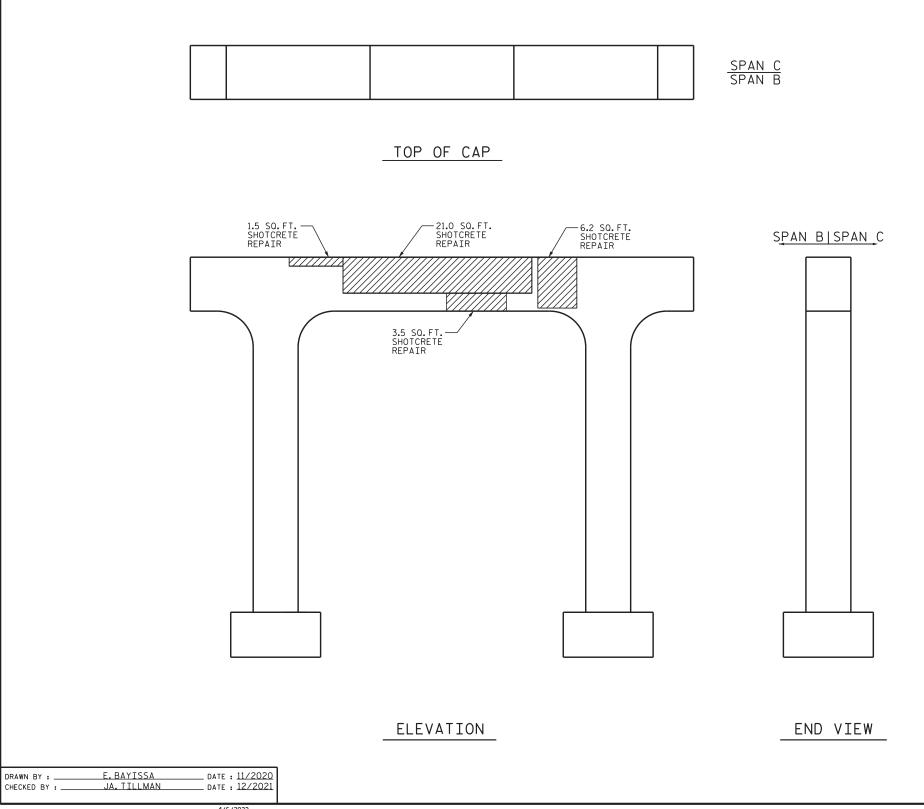


CONCRETE REPAIR AREA



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I	BRIDGE NO	1100)95	
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AS-BUILT REPAIR QUANTITY TABLE					
BENT 2 SPAN B FACE	FCTT		ITIES		
	ESII	MATE	ACI	UAL	
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
CAP	32.2	16.1			
COLUMN	0.0	0.0			
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
CAP	0.0	0.0			
COLUMN	0.0	0.0			
EPOXY RESIN INJECTIO	LIN.FT.	LIN	.FT.		
CAP		0.0			
COLUMN		0.0			

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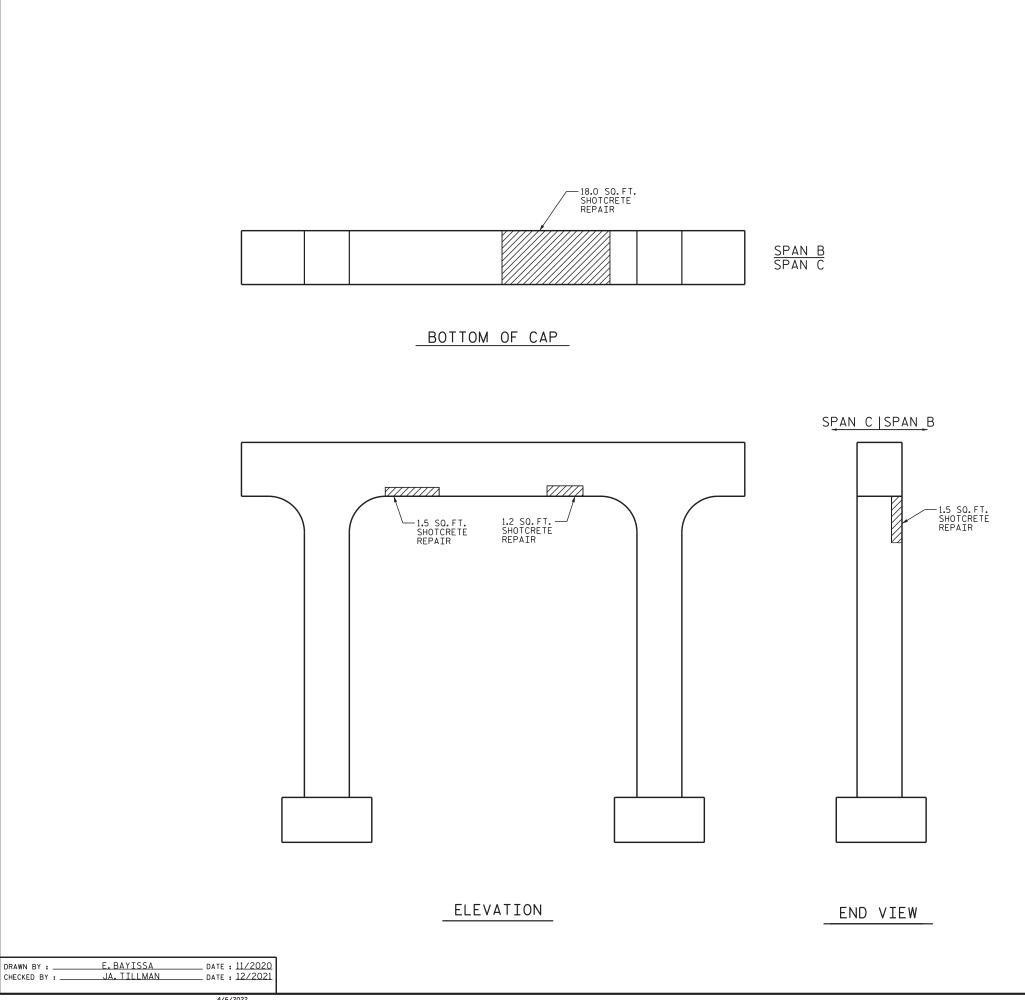
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FOR ADDITIONAL BENT 2 PLANS, SEE SHEETS S-16 THRU S-17.

	SHOTCRETE REPAIR AREA
	CONCRETE REPAIR AREA
	AREA PREVIOUSLY ACCOUNTED FOR ON ADJACENT FACE
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	BRIDGE	NO	110	095	
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SEAL OSIO21 Doussoned br. Martine	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
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AS-BUILT REPAIR QUANTITY TABLE					
BENT 2 SPAN C FACE					
DENT 2 STAR C TAGE	ESTI	MATE	ACT	UAL	
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
CAP	20.7	10.4			
COLUMN	1.5	0.8			
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
CAP	0.0	0.0			
COLUMN	0.0	0.0			
EPOXY RESIN INJECTIO	LIN.FT.	LIN	.FT.		
CAP	P 0.0				
COLUMN		0.0			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

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FOR ADDITIONAL BENT 2 PLANS, SEE SHEETS S-16 THRU S-17.

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SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

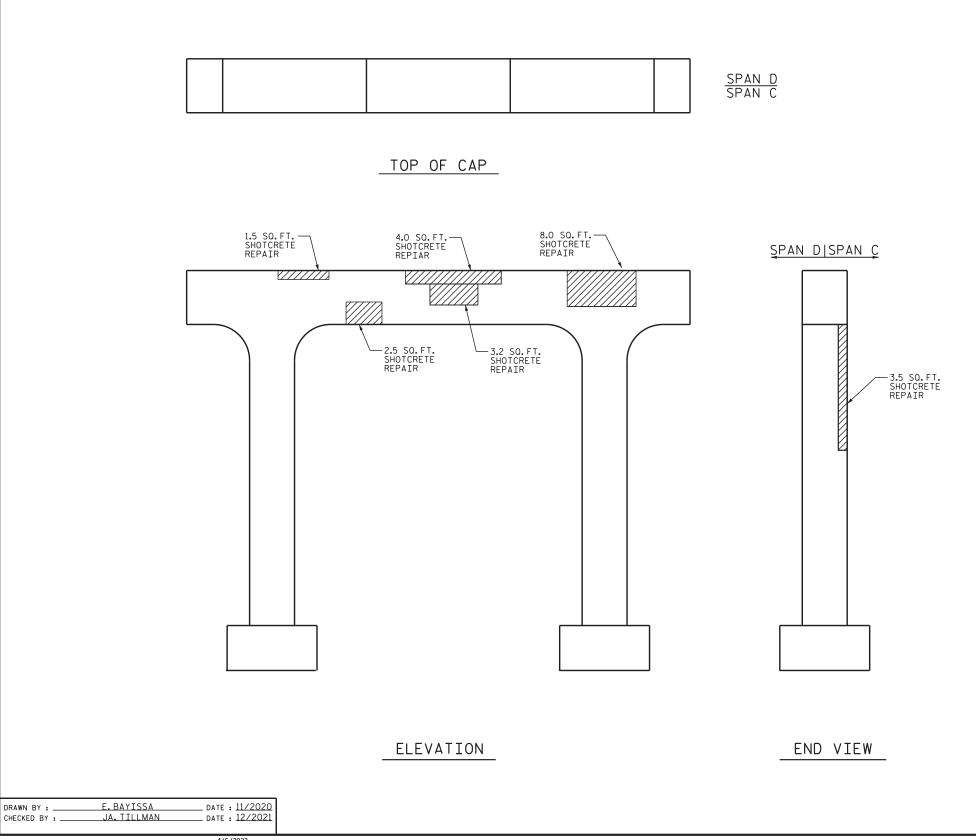


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AS-BUILT REPAIR QUANTITY TABLE						
BENT 3 SPAN C FACE			ITIES			
DENT 5 STAN & TACE	ESTI	MATE	ACT	UAL		
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.		
CAP	19.2	9.6				
COLUMN	3.5	1.8				
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.		
CAP	0.0	0.0				
COLUMN	0.0	0.0				
EPOXY RESIN INJECTIO	LIN.FT.	LIN	.FT.			
CAP		0.0				
COLUMN		0.0				

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE ``TYPICAL CAP AND COLUMN REPAIR DETAILS'' SHEET.

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FOR ADDITIONAL BENT 3 PLANS, SEE SHEETS S-16 THRU S-17.

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SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

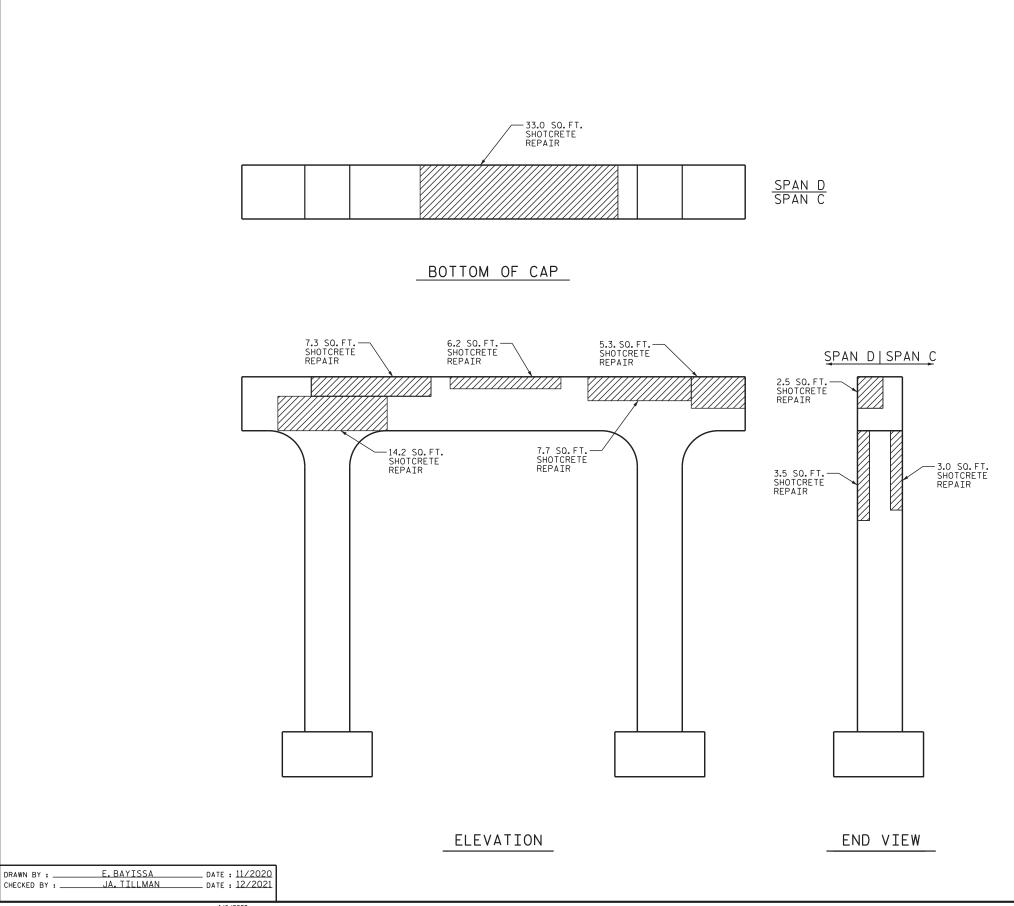




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AREA PREVIOUSLY ACCOUNTED FOR ON ADJACENT FACE

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	SHEET 1 O	- 2			
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AS-BUILT REPAIR QUANTITY TABLE					
BENT 3 SPAN D FACE	ACE QUANTITIES				
SHOTCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
САР	76.2	38.1			
COLUMN	6.5	3.3			
CONCRETE REPAIRS	AREA SQ.FT.	VOLUME CU.FT.	AREA SQ.FT.	VOLUME CU.FT.	
САР	0.0	0.0			
COLUMN	0.0	0.0			
EPOXY RESIN INJECTIO	LIN.FT.	LIN	.FT.		
САР		0.0			
COLUMN		0.0			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MINIMUM OF 1"BEHIND REBAR AND MINIMUM 2"CLEARANCE TO SAWCUT. SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

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FOR ADDITIONAL BENT 3 PLANS, SEE SHEETS S-16 THRU S-17.



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA

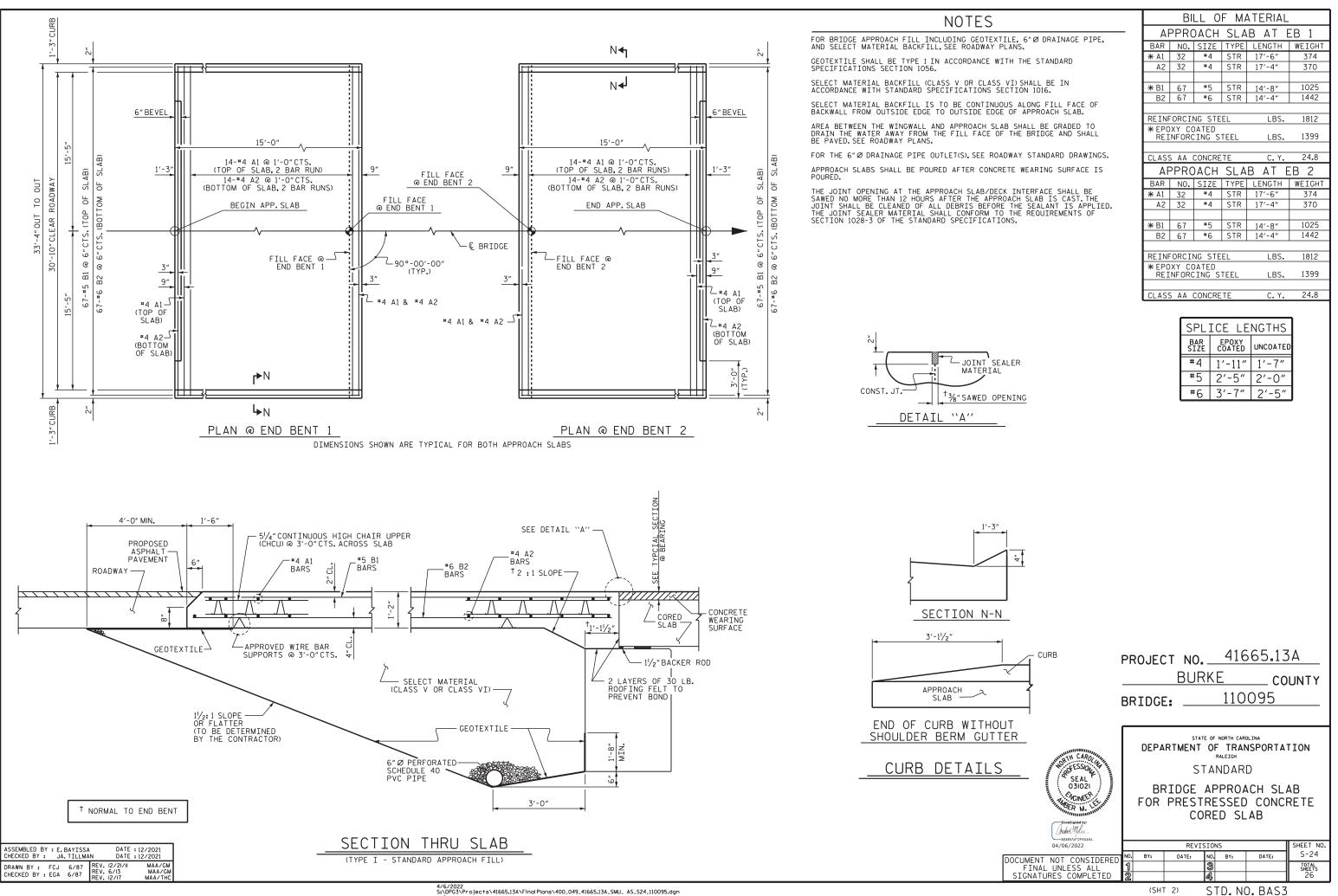


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AREA PREVIOUSLY ACCOUNTED FOR ON ADJACENT FACE

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SEAL OSIO2I	SUBS	B	ENT	E REF	PAIR
THE WORKER M.		SPAI	ND	FACE	
Docusigned by: milite Miller _ Ballos 4=2FAD484.					
04/06/2022		REVIS	IONS		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S-23
FINAL UNLESS ALL	1		3		TOTAL SHEETS
SIGNATURES COMPLETED	2		4		26

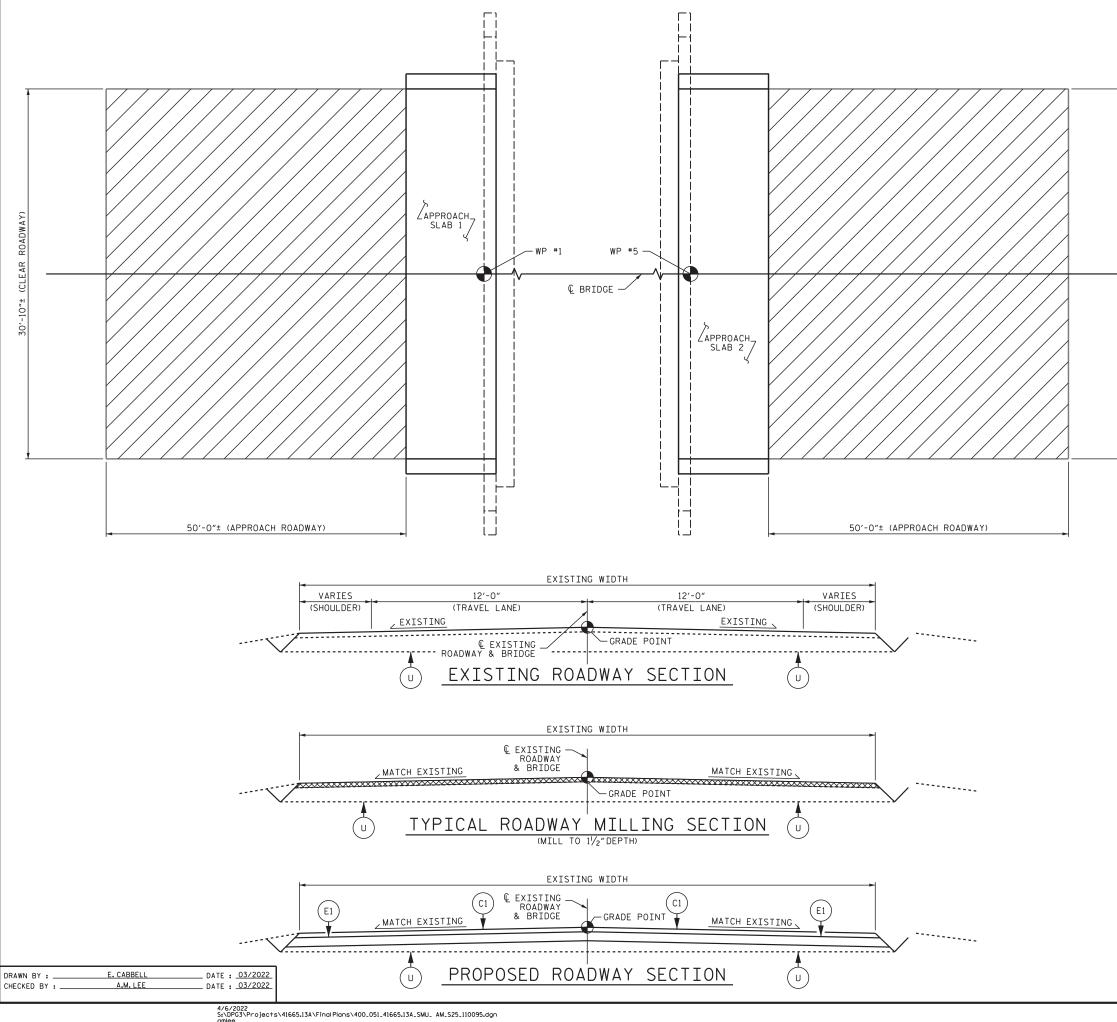


4/6/2022 St.DPC3/Projects/41665.13A/Final Plans/400_049_41665.13A_SMU_ AS_S24_110095.dgn

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-s	BILL OF MATERIAL					
	A	PPR	ОАСН	SLA	BATE	EB 1
EOTEXTILE, 6″Ø DRAINAGE PIPE, DWAY PLANS.	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
NCE WITH THE STANDARD	* A1	32	#4	STR	17'-6″	374
NCE WITH THE STANDARD	A2	32	#4	STR	17'-4"	370
CLASS VI)SHALL BE IN ONS SECTION 1016.	* B1	67	#5	STR	14'-8"	1025
	B2	67	#6	STR	14'-4"	1442
NTINUOUS ALONG FILL FACE OF E EDGE OF APPROACH SLAB.						
	REINF	FORCI	NG STE	EL	LBS.	1812
CH SLAB SHALL BE GRADED TO ACE OF THE BRIDGE AND SHALL		XY CONTROPORT	DATED CING S	TEEL	LBS.	1399
EE ROADWAY STANDARD DRAWINGS.						
LE ROADWAT STANDARD DRAWINGS.	CLASS	S AA	CONCRE	TE	С.Ү.	24.8
CONCRETE WEARING SURFACE IS	AF	PPRC)ACH	SLA	BATE	EB 2
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
AB/DECK INTERFACE SHALL BE E APPROACH SLAB IS CAST.THE	* A1	32	#4	STR	17'-6"	374
BEFORE THE SEALANT IS APPLIED.	A2	32	#4	STR	17'-4"	370
DRM TO THE REQUIREMENTS OF ICATIONS.						
1010.0.	* B1	67	#5	STR	14'-8"	1025
	B2	67	#6	STR	14'-4"	1442
	DETH			E 1	1.00	1010
			NG STE	EL	LBS.	1812
		XY CO	SING S	TEEL	LBS.	1399

SPLICE LENGTHS						
BAR SIZE	EPOXY COATED	UNCOATED				
#4	1'-11"	1'-7"				
# 5	2'-5″	2'-0"				
#6	3'-7"	2'-5"				



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NOTES

INCIDENTAL MILLING - EXISTING APPROACH ASPHALT PAVEMENT TO BE MILLED AS NECESSARY TO ATTAIN MINIMUM 11/2" DEPTH OF NEW ASPHALT PAVEMENT.NEW ASPHALT PAVEMENT SHALL BE OF THICKNESS NECESSARY TO PROVIDE A SMOOTH TRANSITION BETWEEN THE ROADWAY AND THE BRIDGE DECK. THE NEW ASPHALT PAVEMENT THICKNESS MAY EXCEED $1^{1}/_{2}$ " DUE TO SETTLEMENT OF THE EXISTING APPROACH.

SUMMARY OF QUANTITIES						
	ESTIMATE	ACTUAL				
INCIDENTAL MILLING	342.6 SQ. YD.					
ASPHALT CONCRETE BASE COURSE. TYPE B25.0B	60.0 TONS					
ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B	30.0 TONS					
ASPHALT BINDER FOR PLANT MIX	5.0 TONS					
ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B	30.0 TONS					

C1	PROPOSED VARIABLE DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C AT AN AVERAGE RATE OF 112 LBS.PER S0. YD.PER 1" DEPTH.TO BE PLACED IN LAYERS NOT LESS THAN $1/2$ " IN DEPTH OR GREATER THAN 2" IN DEPTH.
E1	PROPOSED VARIABLE DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS.PER SO.YD.PER 1"DEPTH.TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5" IN DEPTH.
U	EXISTING PAVEMENT

PROJECT NO.	41665 . 13A
BURKE	COUNTY
BRIDGE NO.	110095

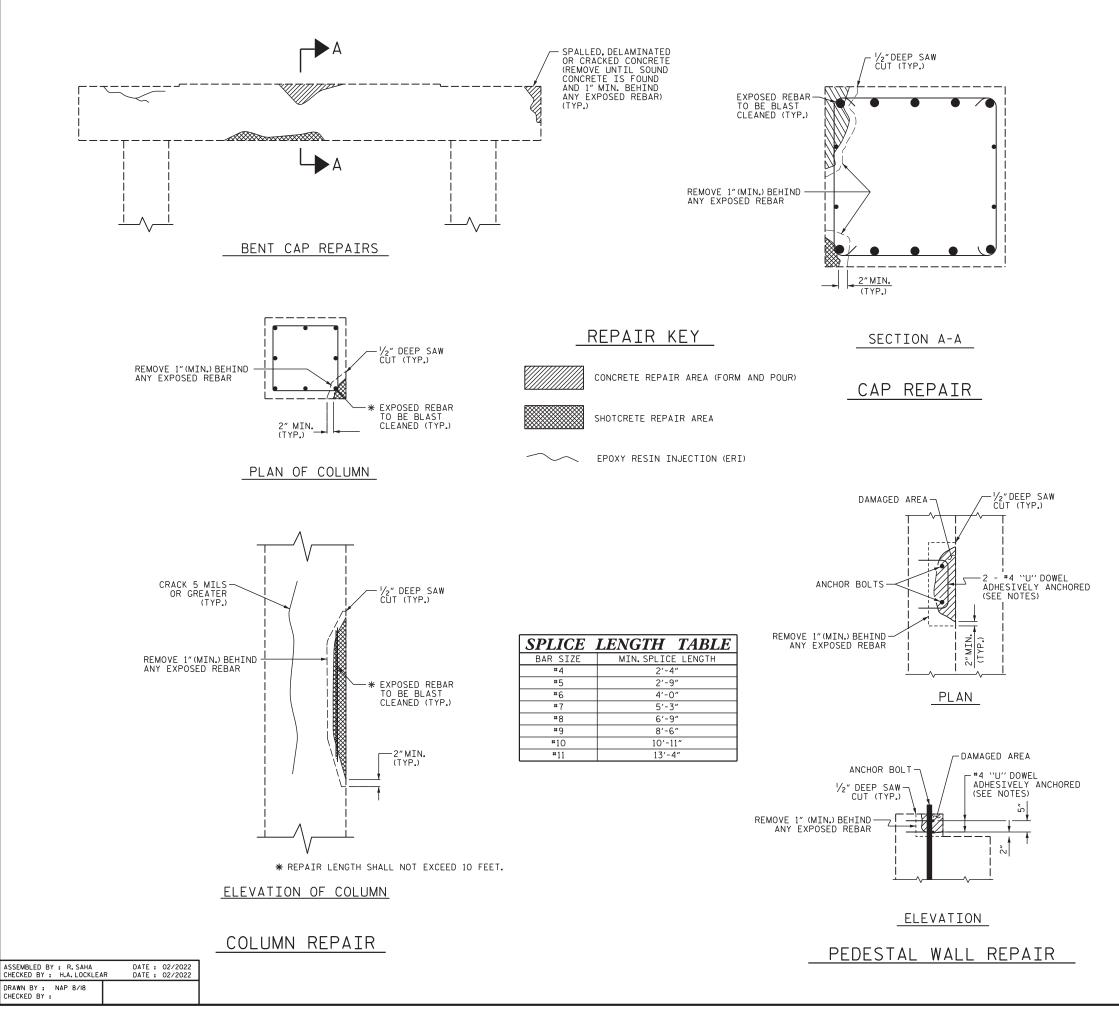
STATE OF NORTH CAROLINA





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	REVISIONS						SHEET NO.
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PLETED	2			4			26



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NOTES

TYPICAL BENT CAP REPAIRS ARE SHOWN.REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REOUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1"BEHIND REBAR AND MINIMUM OF 2"CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CROSS SECTIONAL AREA SHALL BE REMOVED AT ONE TIME.SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR COLUMN, IF THE AREAS OF REMOVAL ARE NOT ADJACENT TO OR DIRECTLY OPPOSITE ONE ANOTHER. IF REMOVAL EXTENDS MORE THAN 1/2"BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

THE #4 ``U'' DOWELS ARE REQUIRED ONLY AROUND THE ANCHOR BOLTS. THE EXISTING REINFORCING STEEL IN THE PEDESTAL WALL SHALL BE CLEANED, STRAIGHTENED AND REMAIN IN PLACE.

FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

COAT ALL REPAIR SURFACE AREAS ON THE TOP OF CAPS, INCLUDING CHAMFERS, WITH EPOXY PROTECTIVE COATING, OVERLAPPING THE REPAIR AREA BY A MINIMUM OF 3" ON ALL POSSIBLE SIDES.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY PROTECTIVE COATING, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION (ERI), SEE SPECIAL PROVISIONS.

	PROJ.NO. <u>41665.</u> <u>BURKE</u> BRIDGE NO. <u>1100</u>					UNTY
SEAL OSIO21	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD TYPICAL CAP AND COLUMN REPAIR DETAILS					
04/06/2022		SHEET NO.				
OCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO.	BY:	DATE:	S-26
FINAL UNLESS ALL	1		3			TOTAL SHEETS
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